



USER' S MANUAL

HelixPile software program (Version 2012)

Version 1.0

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Deep Excavation LLC

www.deepexcavation.com

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Chapter 1: Introduction to HelixPile

1.1 About HelixPile (Helical Pile Engineering Program)

HelixPile 2012 is a user friendly, modern and powerful software program for the design of helical piles. HelixPile allows the user to include an unlimited number of stage conditions and soil profiles. HelixPile incorporates the latest recommendations and reports the controlling design conditions.

1.2 Software Compatibility & Installation

HelixPile is compatible with Windows (OS) XP, Vista and 7. A minimum of 120 Mb must be available on your hard disk.

1.3 Support & Technical Assistance

Support and technical assistance for *HelixPile* is offered through our web site at:

www.deepexcavation.com

1.4 Acknowledgements

Deep Excavation LLC would like to acknowledge the contribution of Howard Perko from Magnum Piering, who has been extremely helpful when the program authors had questions regarding helical pile design.

1.5 End User License Agreement

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Should you have any questions regarding this agreement please email:

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1.5 Activating the software

In order to activate the license, the following steps are required;

- 1) Download and install the software.
 - 2) Keep the SHIFT key pressed (or CAPS locked) and start HelixPile 2012
 - 3) The activation window should appear (Figure 1.5.1).
 - 4) E-mail us the SITE and MID codes that appear in this window (see Figure 1.5.1).
 - 5) We will then e-mail back the user's activation code
 - 6) Restart the program (with CAPS locked) and enter the activation code in the Deep-Paratie activation window (select the option Unlock application) (Figure 1.5.2).
- (please pay attention not to paste the activation code with any additional space characters)
- 7) Press Continue.

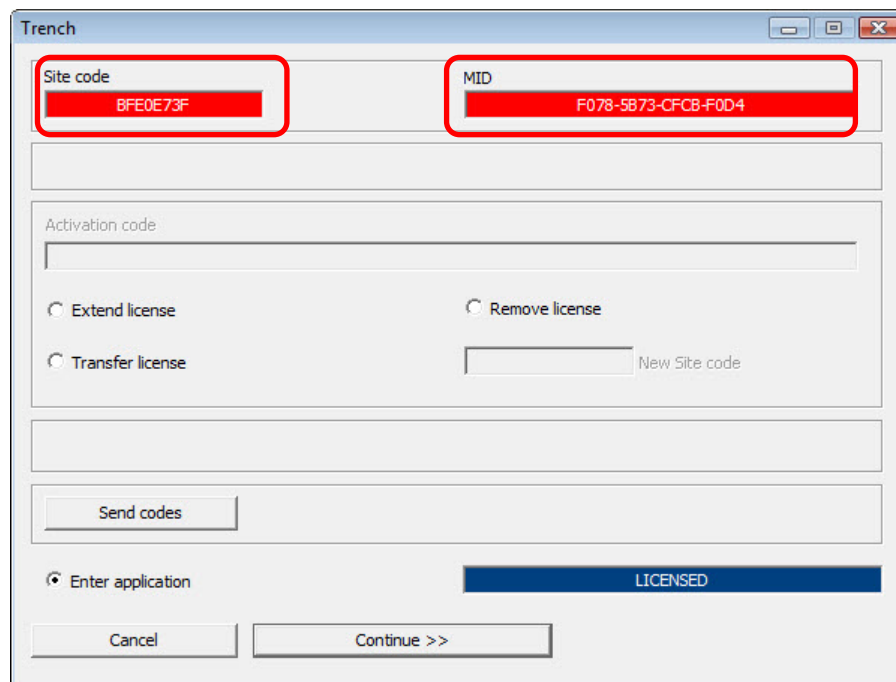


Figure 1.5.1: The HelixPile activation window – SITE and MID codes.

Trench

Site code: BFE0E73F MID: F078-5B73-CFCB-F0D4

Activation code

☐ Extend license ☐ Remove license ☐ Transfer license

New Site code

Send codes

☒ Enter application

Cancel Continue >>

LICENSED

Figure 1.5.2: The activation code area.

Chapter 2: Using HelixPile

2.1 Using HelixPile

HelixPile is a user-friendly software program that includes powerful features and versatile options for helical pile design. HelixPile offers the ability to work with many design sections of a model that can represent various conditions. In a sense, a design section is a design scenario. This way, multiple conditions can be examined simultaneously. The main interface is shown in Figure 2.1.1. The general philosophy in creating a model in HelixPile is:

- 1) Specify the global coordinates.
- 2) Specify the soil types and properties.
- 3) Specify the layers and stratigraphy.
- 4) Create a generalized water table.
- 5) Specify the pile properties (depth, x-coordinate, number and dimensions of plates).
- 6) Specify different stages
- 7) Specify HelixPile analysis combinations and standards.
- 8) Analyze the project.

The general tabs that appear on the top of the program have the following functions.

1. **General:** This tab includes general information about the project, model limits, general settings and the HelixPile analysis options.
2. **Properties:** This tab contains various information about Borings and soils.
3. **Model:** Here we can define borings, surface and water elevations and add or delete stages.
4. **Settings:** Here we can modify the general program settings (units, language options etc).
5. **Design:** In this tab we can define code options and several safety factors.
6. **Results:** In this tab we can select to present results directly to the screen after the analysis is performed.
7. **Report:** In this tab we can select options for generating output reports, or viewing calculation progress files.
8. **View:** In this tab we can modify various view options or generate a top view of the model.

9. Help: This tab provides links to help and terms of use.

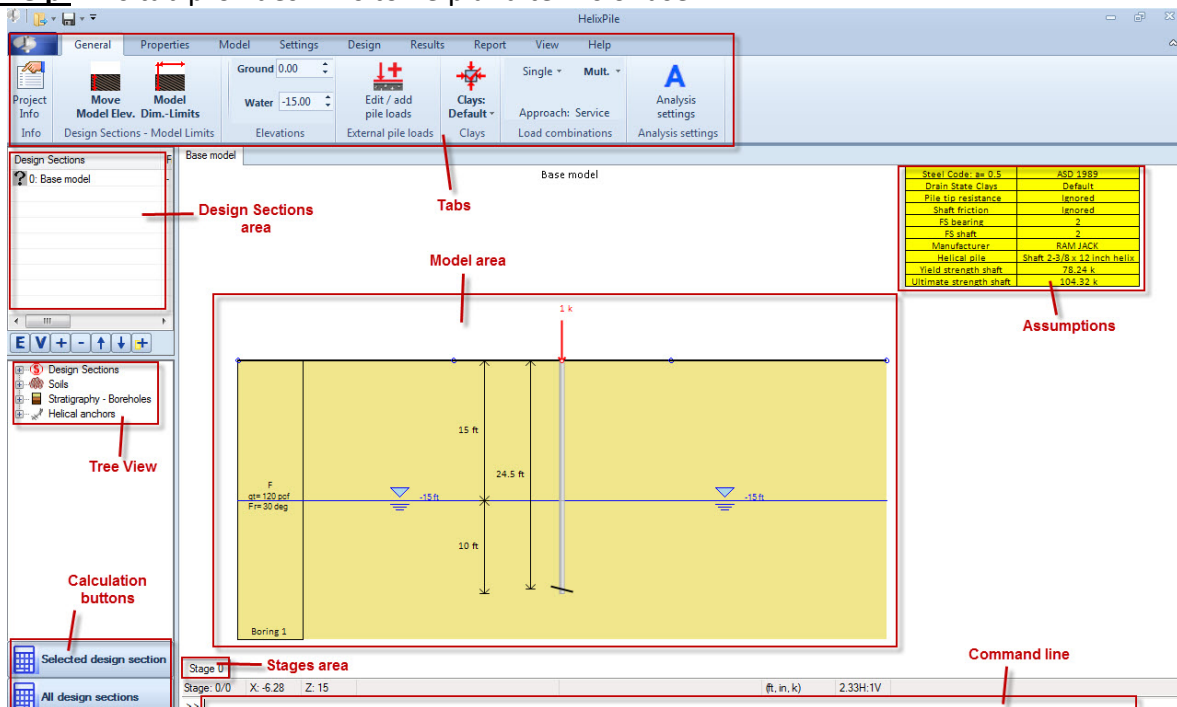


Figure 2.1.1: General HelixPile 2012 Interface

2.2.1 HelixPile Toolbar Functions

The following section provides a detailed list of all toolbar functions. The first tab group to encounter contains the following options:

- **Start button** 

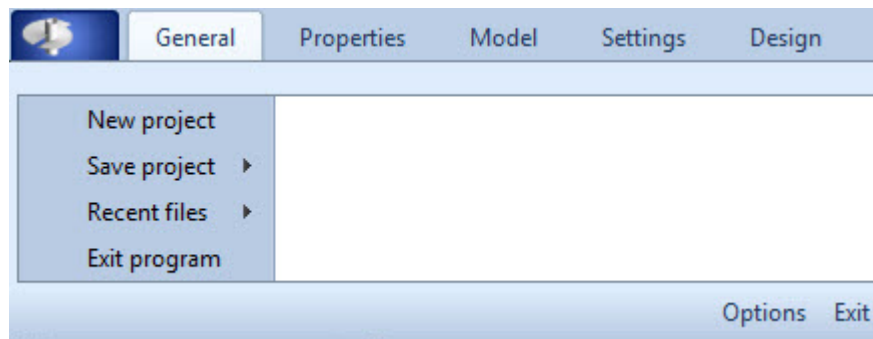









Figure 2.2.1: Main button

This window provides the following options:

- Create a new project
- Save a project to a folder
- See and choose to open recent files
- Exit the program

A horizontal toolbar available right under the design section list allows the user to edit section names, add new design sections etc (Figure 2.2.2). The icons are presented and described in the table below:

Tool	Description
	Edit the name of the selected design section
	Generate a new view of the current design section
	Add a new design section
	Delete design section
	Move design section up on the list
	Move design section down on the list
	Add a new design section (empty – including only stage 0)

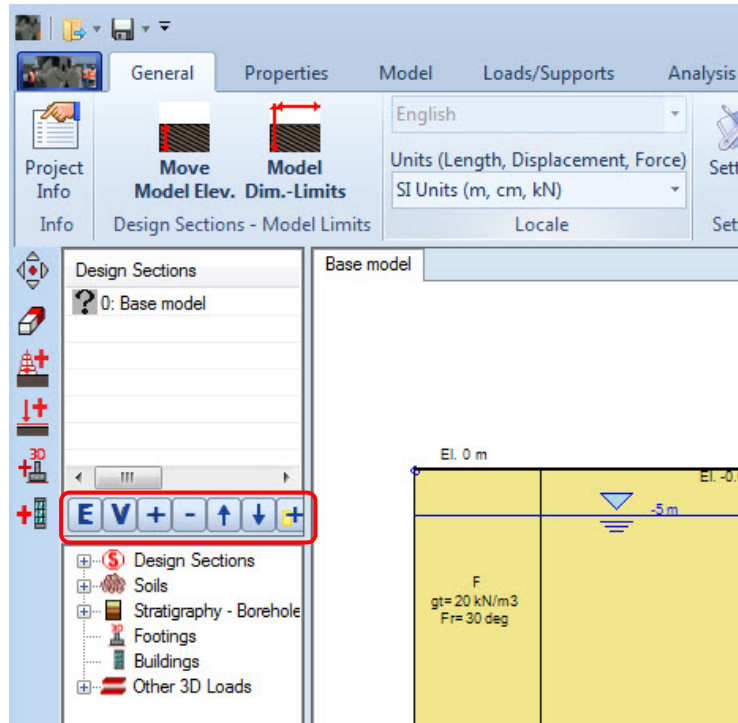




Figure 2.2.2: Design section toolbar.

Two calculation buttons are available on the bottom left corner of the program. By pressing either of these buttons the program will analyze the desired design section and determine the estimated pile capacity.

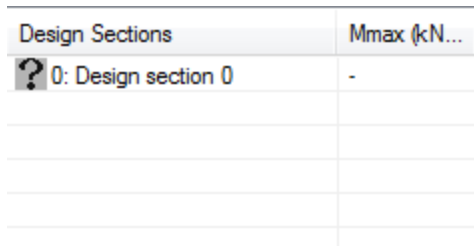
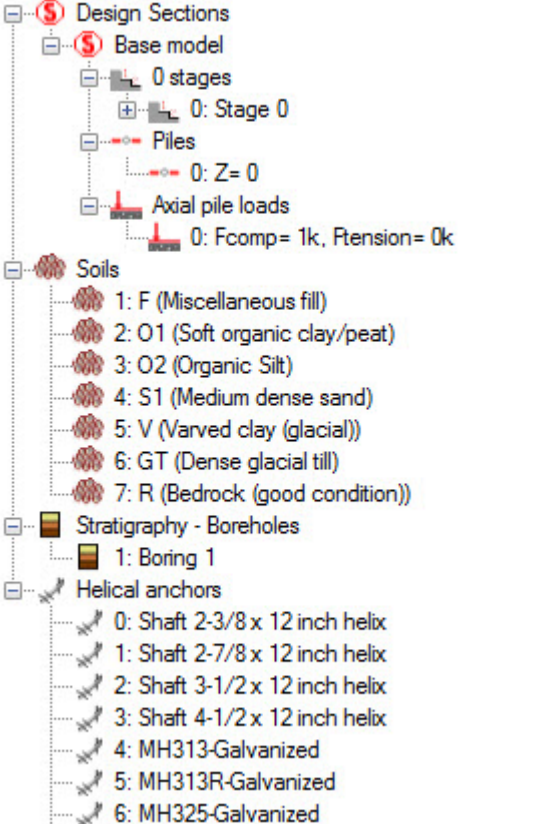


Figure 2.2.3: Calculate tools

Tool	Description
 Selected design	Calculate the selected design section
 All design sections	Calculate all design sections

2.2.2 Design Section List, and Project Tree View

HelixPile includes a table on the left that displays all available design sections and a tree-style project view. The tree view enables the user to quickly access vital project data, as well as visualize crucial project settings. The next table briefly describes the functionality of the design section list, and tree view items.

	<p>Selects current design section, shows available design sections.</p>
	<p>Shows available design sections</p> <p>Pile loads (right click to add or erase) Piles (right click to add or erase)</p>
	<p>Available soil types (by clicking the user can select which soil’s properties to modify)</p>
	<p>Available boreholes (by clicking the user can select which borehole’s properties to modify)</p>
	<p>Helical anchors (by clicking the user can select to change the properties of the helical anchors)</p>

2.3 General menu

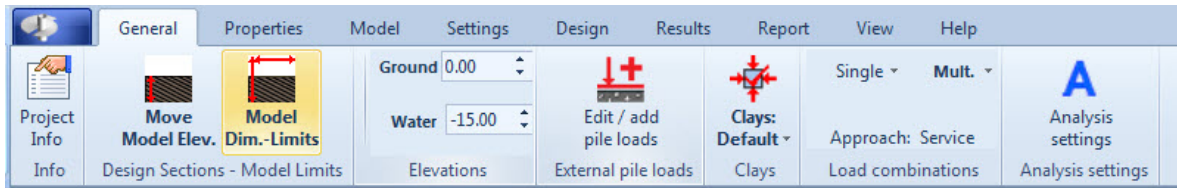
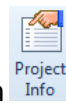
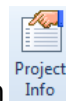


Figure 2.3.1: Project info, model limits, ground and water elevations, load and clay settings, load combinations, and analysis settings.



- **Project Info:** by pressing the button , we can change the project, file, company and engineer name

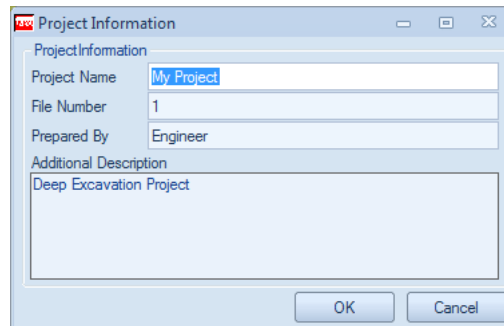


Figure 2.3.2: Project information dialog.



- **Move model elevation:** by pressing the button , we can change the model elevation by entering a new top of wall elevation.

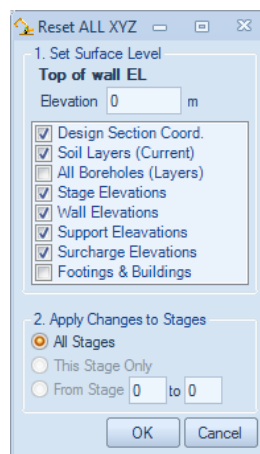



Figure 2.3.3: Model Elevation dialog.

The user can choose the objects to be affected by the change in elevation. These are:

The design section coordinates
Soil layers elevation (of current borehole)
Soil layer elevations for boreholes
Surface elevations
Pile top elevations
Surcharge elevations
Elevations of all footings and buildings

The top of the pile is used as the point of reference for changing all elevations.

- **Model Dimension - Limits:** by pressing the button  , we can change the design section name and the model Limits to create a better view of the model.

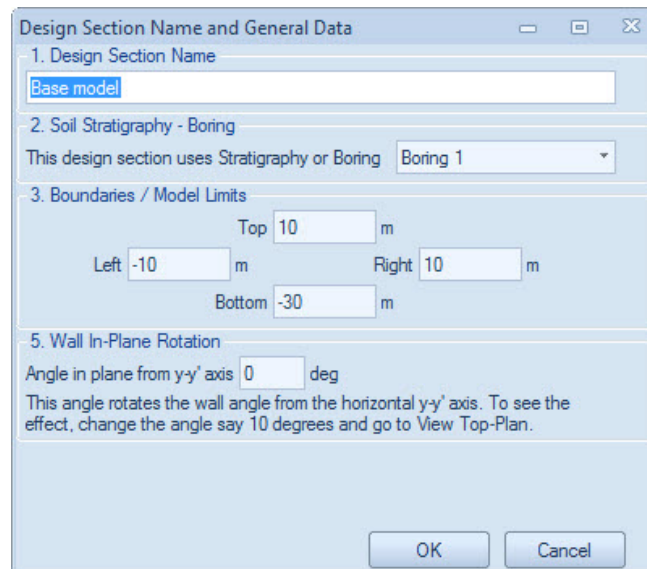


Figure 2.3.4: Model Dimension – Limits dialog.

This dialog includes the following options:

- The design section name.
- The model limits. Here we can define the top, bottom, left and right limits of the model. These are absolute coordinates.
- Angle in plane from y-y' axis.

- **Elevations:** Change the general ground elevation and define the water table.



- **External pile loads:** by pressing the button , the "Loads on Pile" dialog appears.

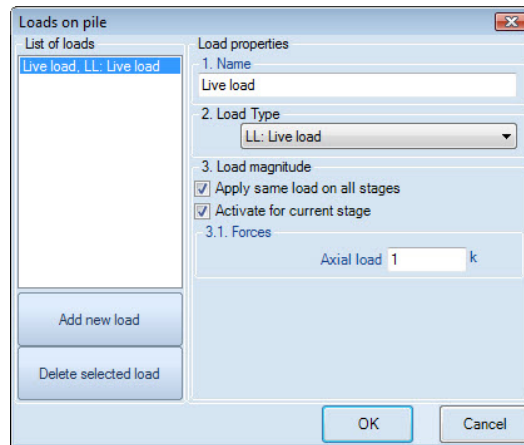


Figure 2.3.5: Loads on Pile dialog

In this dialog we can:

- Add or remove a pile load.
- Specify the load type as presented in the figure below

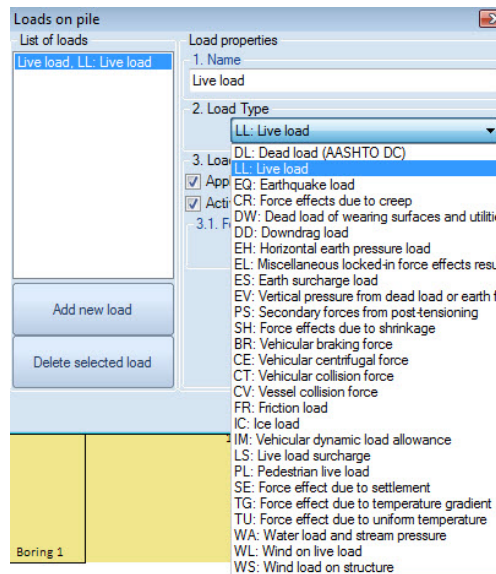


Figure 2.3.6: Settings – Soil/Properties Tab.

We can choose among the following options:

DL	Dead load (AASHTO DC)
LL	Live load
EQ	Earthquake load
CR	Force effects due to creep
DW	Dead load of wearing surfaces and utilities
DD	Downdrag load
EH	Horizontal earth pressure load
EL	Miscellaneous locked-in force effect results
ES	Earth surcharge load
EV	Vertical pressure from dead load of earth fill
PS	Secondary forces from post-tensioning
SH	Force effects due to shrinkage
BR	Vehicular braking force
CE	Vehicular centrifugal force
CT	Vehicular collision force
CV	Vessel collision force
FR	Friction load
IC	Ice load
IM	Vehicular dynamic load allowance
LS	Live load surcharge
PL	Pedestrian live load
SE	Force effect due to settlement
TG	Force effect due to temperature gradient
TU	Force effect due to uniform temperature
WA	Water load and stream pressure
WL	Wind on live load
WS	Wind load on structure

- Choose to apply the same load to all construction stages or vary the load magnitude in each stage.
- Define the load magnitude.

- **Clays:** Here we can define the Clay behavior choosing from the list below.

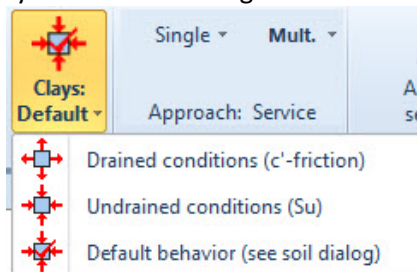


Figure 2.3.7: Clay behavior options.

- **Load combinations:** Here we can select to general a series of load combinations from specified standards such as AASHTO, EC7 etc.

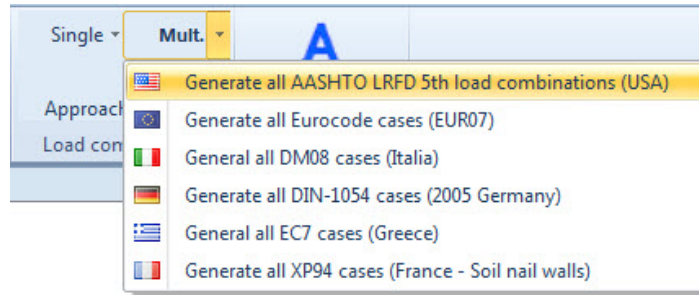
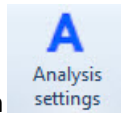
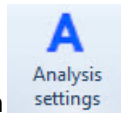


Figure 2.3.8: Load combinations available in HelixPile.



Analysis Settings: By pressing the button , the Helical pile analysis settings dialog appears.

General tab: Here we can define the general analysis settings that control how calculations are performed.

Include shaft resistance: If selected, then shaft side resistance will be included. If we opt to include shaft resistance then the following options become available. Please refer to section 5.2 for more information.

Use limiting effective stress: If selected, then a maximum limiting effective stress is used for the cylinder method, for shaft side resistance, and for bearing capacity calculations.

Include tip resistance: If selected, then the tip resistance will be included in the compression bearing capacity calculations. Tip resistance should be included only when the helical shaft is fully plugged.

Use bearing limit: If selected, then the ultimate bearing pressure at each plate is limited according to the formula $q = f \gamma d (N_q - 1)$ where d is the helix diameter. f is commonly taken as 2 according to recommendations by Perko 2009.

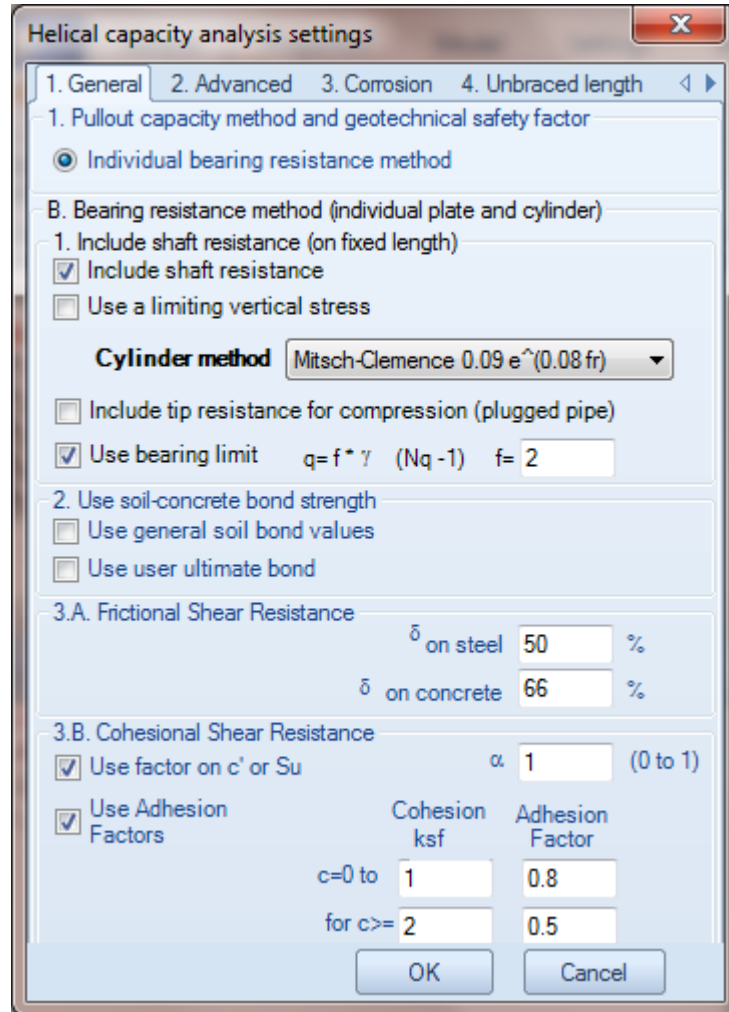


Figure 2.3.9: Helical capacity analysis settings-General tab.

Advanced tab:

- The structural allowable stress factor for determining the structural pile capacity.
- The fixed body and free length colors for the helical piles
- Choose to use custom geotechnical capacity

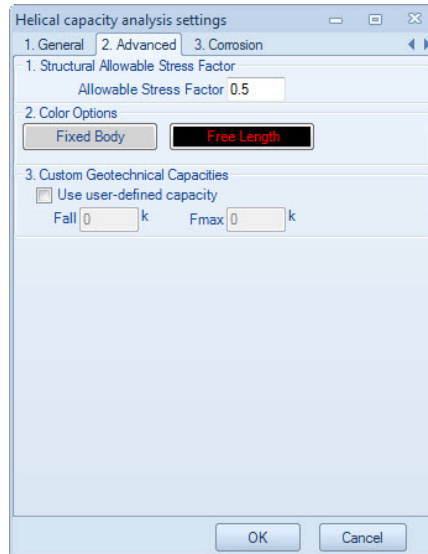


Figure 2.3.9: Helical capacity analysis settings-Advanced tab.

- Corrosion tab: Here we can:
- Specify the design time (or design life) of the pile.
 - Choose the analysis method. We can choose to use the ICC Method AC355 or the AASHTO2004 Method for moderately corrosive soils.

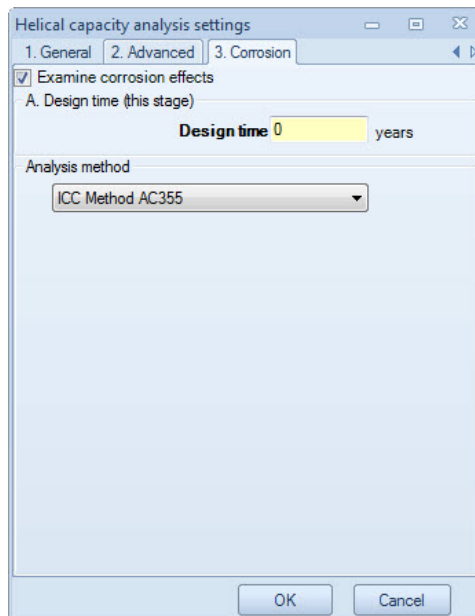


Figure 2.3.10: Helical capacity analysis settings-Corrosion tab.

2.4 Properties menu



Figure 2.4.1: The Properties tab menu.



- **Edit soil type data:** by pressing the button , the soil properties form appears. Here we can add, delete and modify available soils by changing their type, the general properties like unit weights, strength parameters and permeability, modify the elastoplastic parameters and modify the bond resistance for tiebacks. A soil can be used in a boring more than one time. A number of estimation tools that help the user estimate values are also included. **Paragraph 3.3 includes all the options that are available in this form.**

Figure 2.4.2: Edit soil properties dialog.

- **Borings (Soil layers):** by pressing the button , the soil layer dialog appears. In this dialog we can edit the borings available for use in the project. In each boring the user can add soil layers. To do this, we can type the new soil layer's elevation, choose the soil type from the list of soil types and define the new layers OCR and Ko. In addition, by clicking on Edit button, we can modify the selected soil's properties (**see paragraph 3.4**). The coordinates X and Y refer to the plan location of the boring and do not affect analysis results.

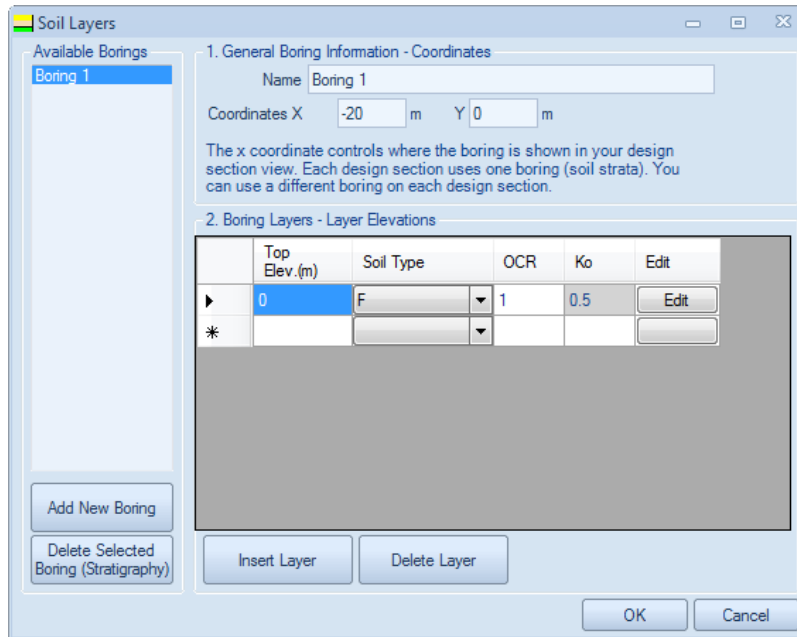
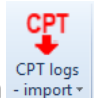


Figure 2.4.3: Edit soil layers dialog.

- **CPT logs:** by pressing the button , we can add borings and soils by using a CPT test results file as performed by Geologismiki CPT. The options of Figure 2.4.4. are available. By choosing a CPT log input file, the following dialog appears:

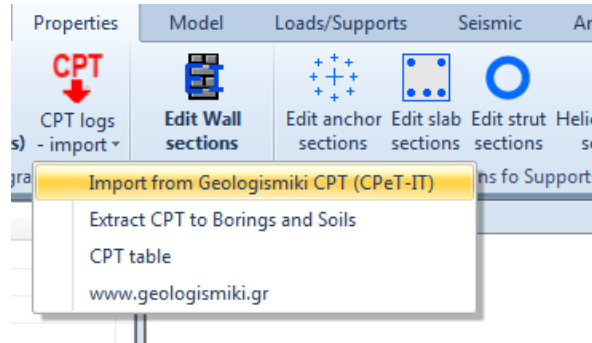


Figure 2.4.4: Available options for CPT logs.

The following options are available:

Import from Geologismiki CPT	Select a CPT file to import
Extract CPT to Borings and soils	Choose to add the soils from CPT log to the model's soils and borings databases
CPT table	This opens the CPT dialog
www.geologismiki.gr	This leads to the site of Geologismiki

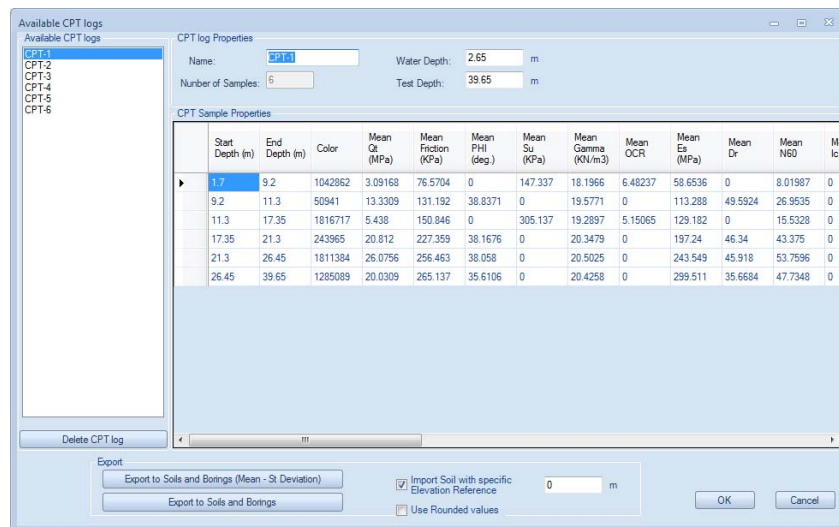
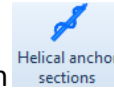


Figure 2.4.5: Available CPT logs dialog.

In this dialog we can export the CPT test borings and soils to the program’s database.



- **Edit helical anchor sections:** By pressing the button we can edit the structural and geotechnical properties of the helical anchor sections (can be used in tiebacks). **The properties that exist on this form and can be modified are described in paragraph 3.5.**

The dialog box 'Helical anchor sections' contains three tabs: A. General, B. Geotechnical capacity options, and C. Advanced. The 'General' tab is active, showing fields for Name, Manufacturer, Tel, web, Shaft dimensions (fy, fu, E, Diameter, Thickness, Area, J, rx), and Helix dimensions (Number of helix plates, End offset, Helix diameter, Effective helix area, Helix spacing, Helix pitch, Helix thickness). It also includes checkboxes for 'Shaft is grouted externally' and 'Use different size plates'. A list of helical sections is on the left, and buttons for 'Add new helical section' and 'Delete selected helical section' are at the bottom left. A checkbox for 'Beam-on-elastic-foundations Option' is at the bottom, with a note about ignoring capacities for spring methods.

Figure 2.4.6: Edit helical anchor sections dialog.

- **Structural materials:** In this area we can edit the structural material properties.

The following options are available (Figure 2.4.7):

Concrete
Steel

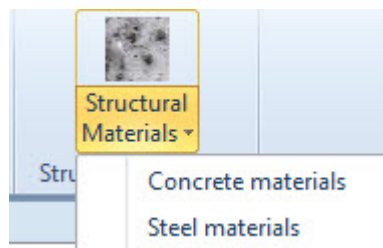


Figure 2.5.9: Structural material options.

- **Edit steel properties:** By pressing the Steel Materials we can edit the structural steel properties. We can import already available materials from the “Import standard steel materials” box.

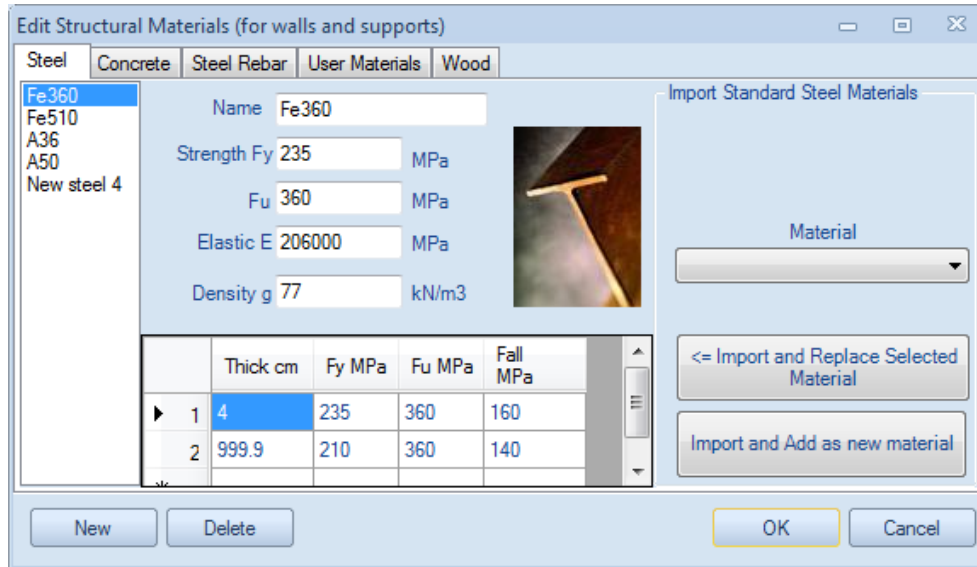


Figure 2.4.10: Edit structural steel properties dialog.

In this form we can define the following properties:

The steel name
The yield strength Fy
The ultimate strength Fu
The modulus of elasticity E
The density g
The steel material used
Import and replace selected material
Import and add as a new material

- **Edit concrete properties:** By pressing the Concrete Materials we can edit the concrete properties. We can import already available materials from the “Import standard concrete materials” box.

Figure 2.4.11 shows the 'Edit Structural Materials (for walls and supports)' dialog box. The 'Concrete' tab is active. The 'Name' field is 'C20/25'. The 'Strength Fc' is '20' MPa, 'Elastic E' is '29962' MPa, 'Density g' is '25' kN/m3, and 'Tension Strength Ft' is '10' (% Fc). The 'Import Standard Concrete Materials' section has a 'Reference Standard' dropdown set to 'None' and a 'Material' dropdown set to 'C8/10'. There are buttons for 'Import and Replace Selected Material' and 'Import and Add as new material'. At the bottom are 'New', 'Delete', 'OK', and 'Cancel' buttons.

Figure 2.4.11: Edit concrete properties dialog.

In this form we can define the following properties:

The steel name
The concrete strength Fc
The tension strength Ft (% of compressive strength)
The modulus of elasticity E
The density g
The standard concrete material reference standard
The concrete material
Import and replace selected material
Import and add as a new material

2.5 Model menu

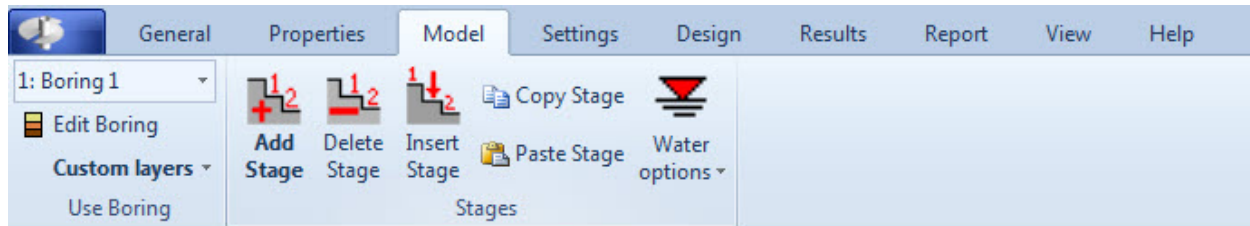


Figure 2.5.1: The Model tab menu.

- **Custom layers:** by pressing the button **Custom layers**, we can choose to use the Custom layer mode and use non parallel soil layers. In this mode, a model will not use a borehole to define soil layers. Instead, the user can define arbitrary layer lines from left to right and create non-horizontal soil layer conditions.

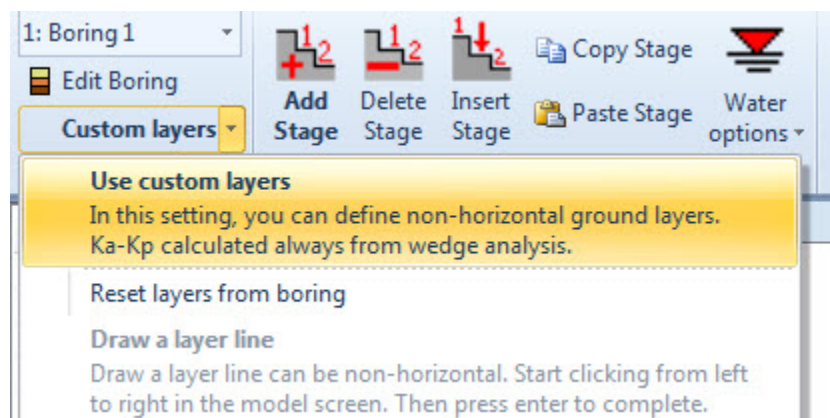






Figure 2.5.2: Custom layers mode.

- The option Use custom layers switches on the custom layer mode.
- The reset layers from boring option, resets all custom layer lines to horizontal using the boring applicable for the current design section.
- The draw layer line tool enables the user to draw a layer line by clicking in the model (left to right). Press enter to complete.

- **Stages:** In this area we can add, delete, insert or copy a construction stage.

Icon	Description
	Add a new construction stage
	Deletes the current construction stage
	Insert a construction stage after the current stage
Copy Stage	Copy selected construction stage
Paste Stage	Paste construction stage

By clicking on the button , the Ground water table dialog shows up.

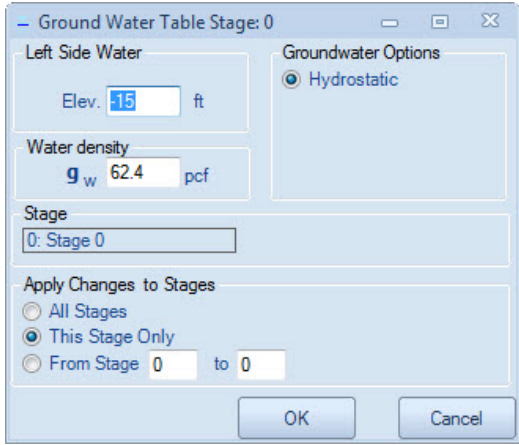


Figure 2.5.3: Ground water table.

The following table presents the options that are included in the ground water table dialog.

Define the general water elevation
Define the water density γ_w

2.6 Settings menu

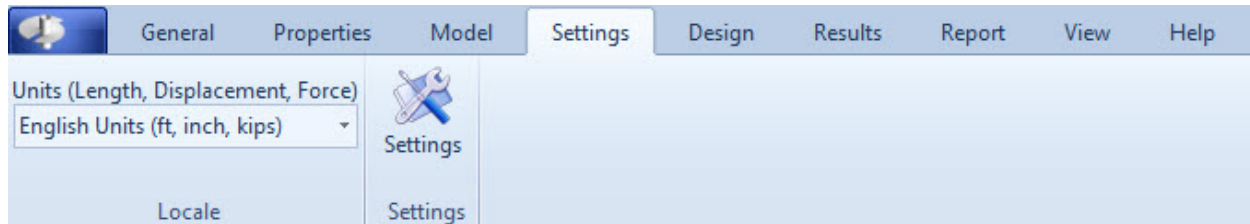



Figure 2.6.1: The Settings tab menu.

- **Locale:** here we can define the units that will be used in this model.
 - **Settings:** by pressing the button , the Default Settings dialog appears.
- General tab: In this tab we can define the default units, company and engineer name and the Auto save directory.

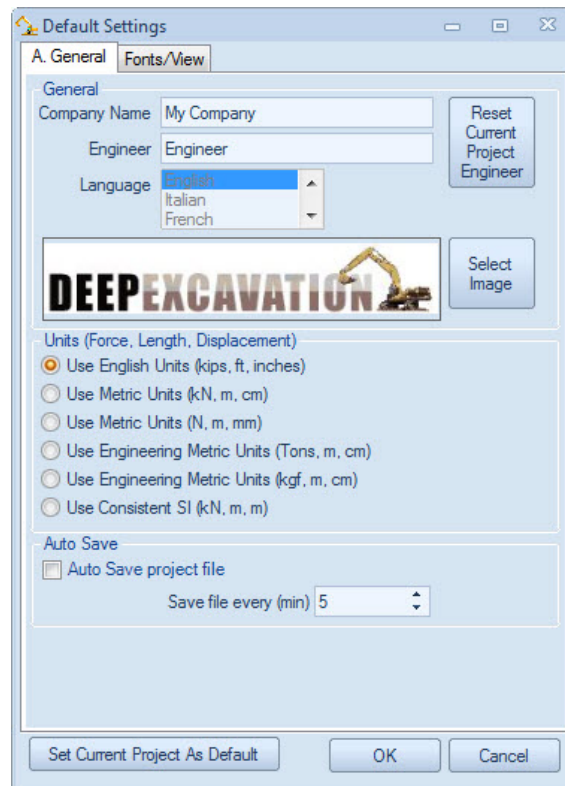


Figure 2.6.2: Settings – General Tab.

- **Fonts/View tab:** In this tab we can define the fonts and some other viewing options.

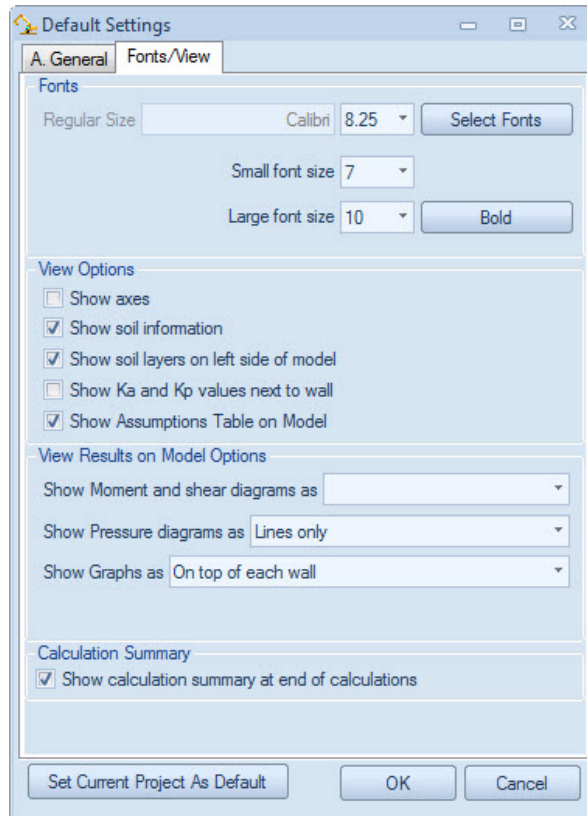


Figure 2.6.3: Settings – Fonts/View Tab.

The available view options are:

- Show axes.
- Show soil information.
- Show soil layers on left side of model.
- Show Ka and Kp values next to the wall.
- Show assumptions table on model.

Furthermore, here we can define some view options regarding the result presentation.

2.7 Design menu



Figure 2.7.1: The Design tab menu.


- **Code options:** By clicking on the button , we can define which structural code’s settings to apply in analysis. These code settings control structural codes and other options.



Figure 2.7.2: Code options.

The following options are available:

Use Eurocode 2, 3 settings	Apply Eurocode settings to the design
Use US allowable settings	Apply US allowable settings to the design
Use US LRFD settings	Apply US LRFD settings to the design
Use AASHTO (US) LRFD settings	Apply AASHTO LRFD settings to the design
Italian DM08 Settings	Apply Italian DM08 settings to the design

- **Safety factors:** Here we can define several safety factors to be used in the design. We can define the Shaft FS, the bearing capacity FS, as well as a custom structural FS.
- **Structural factors:** Here we can define load factors that are applied when an ultimate steel code is used for buckling. Please refer to section 5.4 for more information.

2.8 Results menu

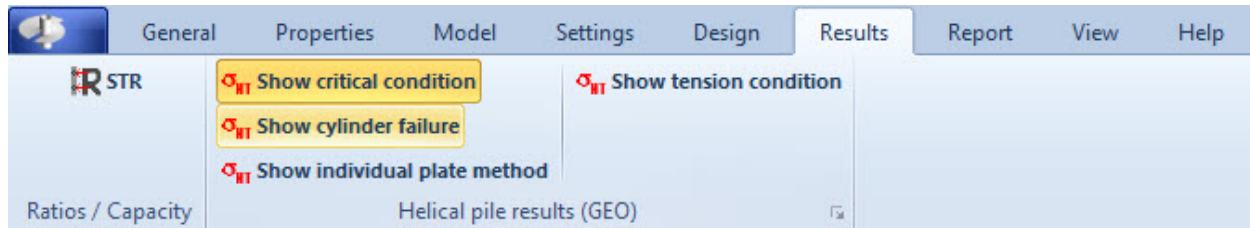


Figure 2.8.1: The Results tab menu.

This tab contains a list of results that can be either viewed on the model or presented in diagrams or tables. Results can be presented when the analysis has been completed.

The following options are provided:

- Show the structural ratios on screen.
- Show the critical condition results on screen
- Show the cylinder failure results on screen
- Show the individual plate results on screen
- Show the results for tension condition on screen

2.9 Report menu

From the Report tab we can control options for viewing reports in pdf or word formats as well as see summary tables of all calculations.

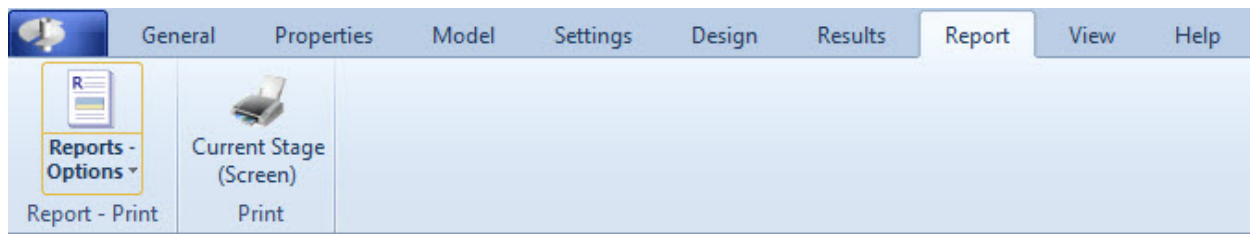



Figure 2.9.1: The Reports tab menu.

Option to create a quick report
Option to create a quick report and include slope stability
Option to create a only stage graphs report
Option to create a typical report

- **Report - print:** By pressing the button , the Reports dialog appears. In this dialog we can choose what parts of calculations and results we want to be included in the reports and the type of file to be exported.

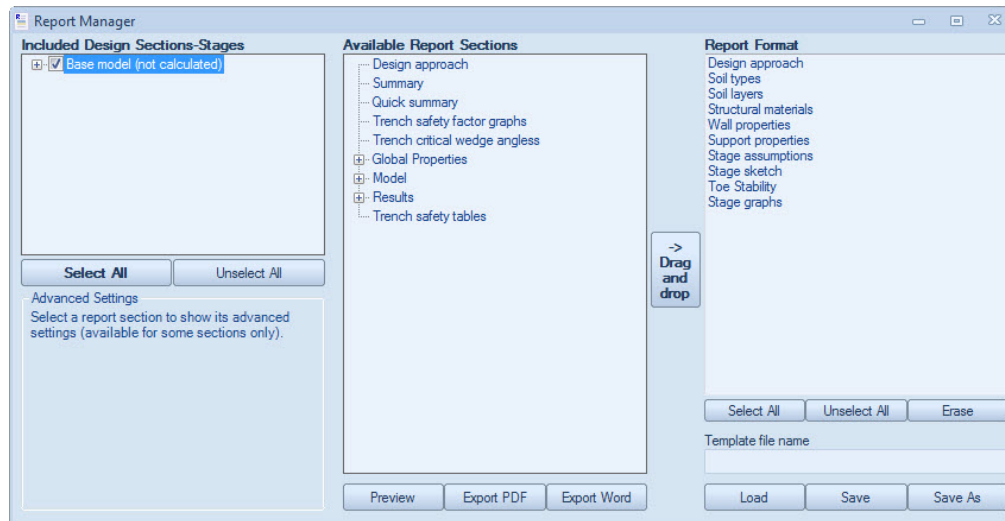


Figure 2.9.2: The Report dialog.

2.10 View menu

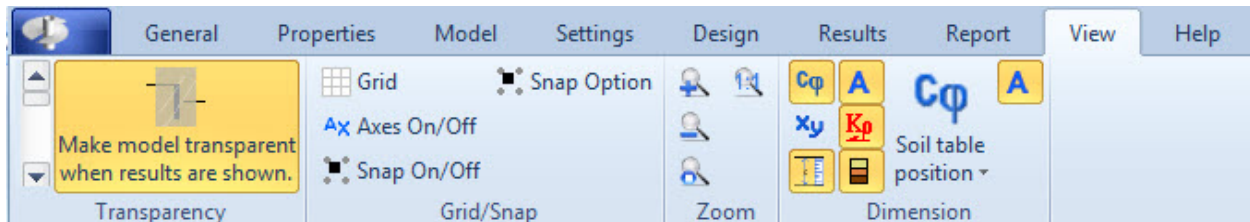
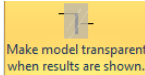




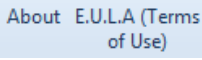
Figure 2.10.1: The View tab menu.

- **Transparency:** By pressing the button , we can make the model transparent when results are shown. The transparency can be adjusted from the vertical bar.
- **Result legend:** By pressing the button , we can turn the result legend on or off.
- **Grid/snap:** By pressing the button , we can change the view options of the snap and axis or show a grid.
- **Zoom and Dimension:** These icons are explained in Chapter 2.1.1.

2.11 Help menu



Figure 2.11.1: The Help tab menu.

- **About and terms of use:** By pressing the button , the user can read the terms of use of HelixPile.

Chapter 3: Data Entry

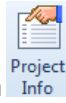
3.1 Data entry: General

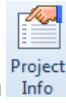
HelixPile is a user-friendly software program and includes powerful features and versatile options. In HelixPile we can work with many design sections of an excavation. In a sense, a design section is a design scenario. This way, multiple conditions can be examined simultaneously. The main interface is shown in Figure 2.1.1. The general philosophy in creating a model in HelixPile is:

- 1) Specify the global coordinates.
- 2) Specify the soil types and properties.
- 3) Specify the layers and stratigraphy.
- 4) Create a generalized water table.
- 5) Specify the pile properties (pile depth, pile x-coordinate, pile section).
- 6) Specify different stages
- 7) Specify HelixPile load combination.
- 8) Analyze the project.

This Chapter describes the data that should be entered by the user in order to create a proper model.

3.2 Data entry: Project information



By clicking on the button , the Project Information dialog appears (Figure 3.2.1). In this dialog we can specify the Project Name, file number (or job number) and the name of the engineer preparing the analysis.

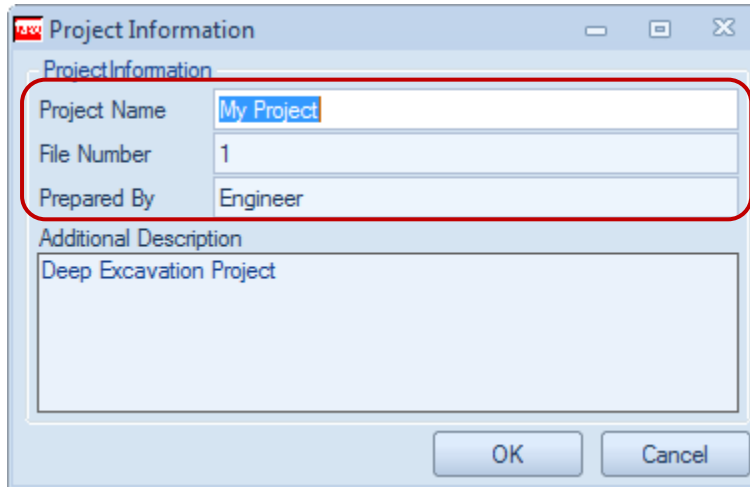
A screenshot of the 'Project Information' dialog box. The dialog has a title bar with 'Project Information' and standard window controls. Inside, there's a section titled 'Project Information' with three input fields: 'Project Name' (containing 'My Project'), 'File Number' (containing '1'), and 'Prepared By' (containing 'Engineer'). These three fields are grouped together and highlighted with a red rectangular border. Below these fields is a larger text area labeled 'Additional Description' containing the text 'Deep Excavation Project'. At the bottom right of the dialog are 'OK' and 'Cancel' buttons.

Figure 3.2.1: Project information dialog.

3.3 Data entry: Soil Data

By pressing the Edit soil type data button of the Properties tab, the Soil Types dialog appears. Here the user can create as many soil types as needed and define their properties. First the user should select to add a new soil type, or choose one from the list in order to modify its properties (Figure 3.3.1).

Figure 3.3.1: Add a new soil or select a soil to modify.

We can change the basic soil information such as soil name, preview color and soil type. The available soil types are Sand, Clay, Silt and Rock. If the soil is clay, a drained or undrained behavior of the clay can be defined at this point. All these details can be modified in the area presented in Figure 3.4.2. Next, the user can modify the general soil properties in the area marked in Figure 3.4.3. The basic properties are described in the table below.

Symbol	Description
γ_t	Total unit weight of soil (used below the water table)
γ_{dry}	Dry unit weight of soil (used above the water table)
c'	Effective soil cohesion
S_u	Undrained shear strength (used for clays when undrained modeling is selected). In the non-linear analysis this is used as an upper limit strength
ν	Poisson's ratio (used for loads calculated with theory of elasticity)
Φ'	Effective soil friction angle
Φ_{cv}'	Constant volume effective shearing soil friction angle used in the non-linear analysis for clays
Φ_{peak}'	Peak effective soil friction angle used in the non-linear analysis for clays
K_x	Soil permeability at horizontal direction
K_z	Soil permeability at vertical direction
K_{oNC}	Coefficient of at-rest lateral earth pressures for normally consolidated conditions
n_{OCR}	Exponent for calculating K_o with $K_o = K_{oNC} * [(OCR)^{(n_{OCR})}]$

The screenshot shows the 'Soil Types' dialog box. On the left is a list of soil types: F, O1, O2, S1, Clay, GT, and Rock. The main area is titled '1. Name and Basic Soil Type' and contains the following fields and options:

- Soil Name:** F
- Description:** Fill
- 2. Soil Type - Behaviour:** Radio buttons for Sand (selected), Silt, Clay, and Rock.
- 3. Default drained-undrained behavior for clays (See Theory Manual):** Radio buttons for Undrained behaviour and Drained.
- A. General** (selected) / **E. Adv.**
- 4. Unit Weights - Density:** γ_t 20 kN/m³, γ_{dry} 19 kN/m³, γ 10
- 5. Strength Parameters and Poisson Ratio:** c' 0 kPa, ϕ' 30 degrees, S_u 0 kPa, $\phi_{cv'}$ Omittec degrees, ν 0.35, $\phi_{peak'}$ Omittec degrees.
- 6. Permeability:** K_x 0.0001 m/sec, K_z 0.0001 m/sec.
- 8. At-rest coefficients:** K_{oNC} 0.5, $nOCR$ 0.5. Below this is the formula $K_o = K_{oNC} * (OCR)^{nOCR}$.

Buttons at the bottom include 'Add New Soil', 'Copy Soil', 'Delete Selected Soil', 'Paste Soil', 'OK', and 'Cancel'. A 'Show test data (SPT, CPT, Etc)' button is also present.

Figure 3.3.2: Define basic soil information.

This screenshot shows the same 'Soil Types' dialog box, but with the '5. Strength Parameters and Poisson Ratio' section highlighted by a red box. The values in this section are:

- c' : 0 kPa
- ϕ' : 30 degrees
- S_u : 0 kPa
- $\phi_{cv'}$: Omittec degrees
- ν : 0.35
- $\phi_{peak'}$: Omittec degrees

The other sections and fields remain the same as in Figure 3.3.2.

Figure 3.3.3: Define main soil data.

If the user presses the Show Test Data button the dialog expands to the right. Here are available tools for estimating main soil data properties from in-situ tests. Data for standard penetration tests, cone penetrometer tests and pressuremeter tests are included (Figures 3.3.4 and 3.3.5).

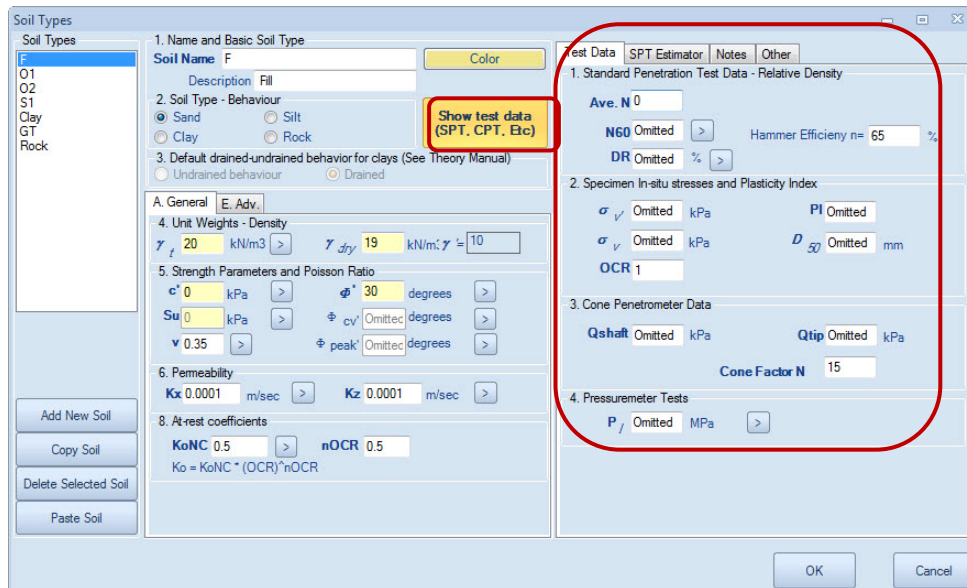


Figure 3.3.4: Test Data.

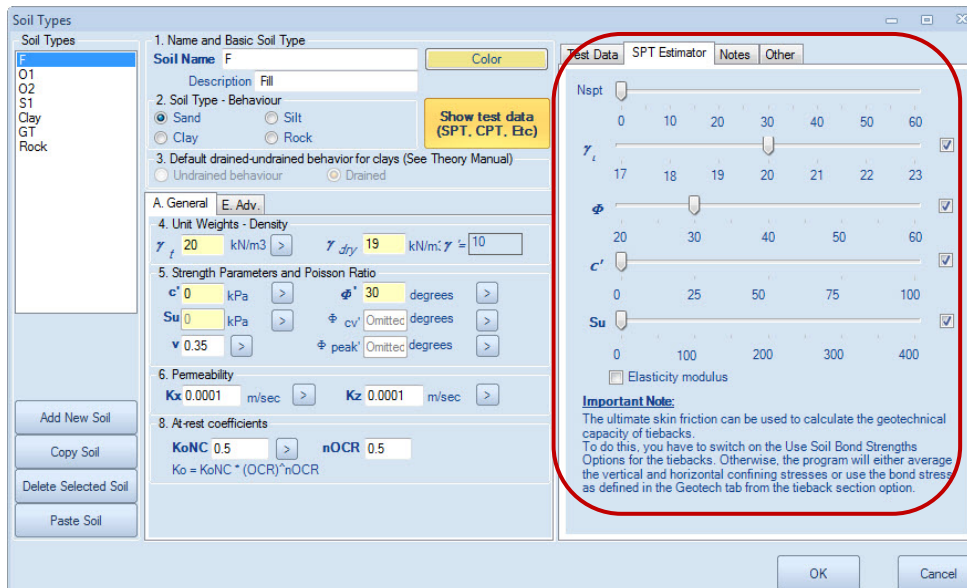



Figure 3.3.5: SPT Estimator.

In addition, next to the main data there is the symbol . This symbol opens a list of tools that can help the user estimate the soil data values according to published literature.

3.4 Data entry: Soil Layers



By pressing the Borings button of the Properties tab, the Soil Layers dialog appears. Here we add or remove soil layers, specify their top elevation and the soil type to be used. In addition, we can define the exact position of the boring on the screen. This dialog also appears if the user double-clicks on the boring, or if he selects it from the tree view. All these are presented in Figure 3.4.1. The OCR (over consolidation ratio) can be defined herein. The user can include as many borings as desired. Each independent design section can include a different boring.

Soil Layers

Available Borings

Boring 1

1. General Boring Information - Coordinates

Name Boring 1

Coordinates X -20 m Y 0 m

The x coordinate controls where the boring is shown in your design section view. Each design section uses one boring (soil strata). You can use a different boring on each design section.

2. Boring Layers - Layer Elevations

	Top Elev.(m)	Soil Type	OCR	Ko	Edit
▶	0	F	1	0.5	Edit
*					

Add New Boring

Delete Selected Boring (Stratigraphy)

Insert Layer

Delete Layer

OK

Cancel

Figure 3.4.1: The soil layers dialog.

3.5 Data entry: Helical Piles

From the Helical Anchors dialog (Figures 3.5.1 and 3.5.2) the user can select an anchor from the existing database of anchors or add some sections to the database. These anchors can use in the project. The General tab contains basic structural information. The Geotechnical capacity tab contains options for calculating the geotechnical tieback capacity. The properties included in this dialog are presented and described in the table below.

HelixPile 2012 has already helical sections from Magnum and Ramjack incorporated (Figure 3.5.1).

Symbol/Option	Description
Fy	Tensile Yield Strength of anchor
Fu	Tensile Ultimate Strength of anchor
Diameter	Anchor diameter
Thickness	Anchor thickness
Ixx	Moment of inertia
Sxx	Elastic section modulus
Zxx	Plastic section modulus
Telastic	Torsional elastic pipe capacity
Tplastic	Torsional plastic pipe capacity
E	Modulus of elasticity
Apipe	Area of the pipe of the anchor
Qyield	Tensile yield shaft capacity
Qultimate	Tensile ultimate shaft capacity
Helix diameter	The diameter of the helical plate
Helix spacing	The spacing between the helical plates
Helix thickness	The thickness of the helical plate
Effective helix area	The effective area of the helical plate
Helix pitch	The helical plate inclination
Qhelix	Ultimate tension capacity for one helical plate
FS	Factor of safety
σ'_{vmax}	Limiting vertical stress

Helical anchor sections

Helical sections

- Shaft 2-3/8 x 12 inch helix
- Shaft 2-7/8 x 12 inch helix
- Shaft 3-1/2 x 12 inch helix
- Shaft 4-1/2 x 12 inch helix
- MH313-Galvanized
- MH313R-Galvanized
- MH325-Galvanized
- MH325R-Galvanized
- MH425-Galvanized
- MH425R-Galvanized
- MH431-Galvanized
- MH431R-Galvanized
- MH625-Galvanized
- MH625R-Galvanized
- MH637-Galvanized
- MH637R-Galvanized
- MH646-Galvanized
- MH646R-Galvanized

Add new helical section

Delete selected helical section

RAM JACK

MAGNUM

A. General B. Geotechnical capacity options

1. Name

Shaft 2-3/8 x 12 inch helix Manufacturer: RAM JACK

Tel: Not specified web: www.ramjack.com

2. Shaft-pipe dimensions and properties

fy 60 ksi fu 270 ksi E 29000 ksi

Diameter 2.375 in Ixx 0.784 in⁴ Torsional pipe capacity

Thickness 0.19 in Sxx 0.66 in³ Telastic 4.13 k-ft

Area Apipe 1.304 in² Zxx 0.909 in³ Tplastic 4.47 k-ft

J 1.568 in⁴ Tensile shaft capacity

rx 0.755 in Qyield 78.24 k

Qultimate 104.32 kip

3. Helix dimensions and properties

Number of helix plates (recommend 1 to 6) 1 End offset 0.5 ft

☐ Use different size plates

Helix diameter 12 in Effective helix area Ah 0.755 ft² Qhelix

Helix spacing 3 ft Helix pitch 3 in Ultimate tension capacity

Helix thickness 0.375 in For one helix

87 k

OK Cancel

Figure 3.5.1: Helical anchor sections dialog-General tab.

Helical anchor sections

Helical sections

- Shaft 2-3/8 x 12 inch helix
- Shaft 2-7/8 x 12 inch helix
- Shaft 3-1/2 x 12 inch helix
- Shaft 4-1/2 x 12 inch helix
- MH313-Galvanized
- MH313R-Galvanized
- MH325-Galvanized
- MH325R-Galvanized
- MH425-Galvanized
- MH425R-Galvanized
- MH431-Galvanized
- MH431R-Galvanized
- MH625-Galvanized
- MH625R-Galvanized
- MH637-Galvanized
- MH637R-Galvanized
- MH646-Galvanized
- MH646R-Galvanized

Add new helical section

Delete selected helical section

RAM JACK

MAGNUM

A. General B. Geotechnical capacity options

3. Estimate geotechnical capacity

A. Torque rating method

Torque rating (by manufacturer)

4 k-ft

Torque installation factor kT

10 1/k >

With this method the ultimate geotechnical pullout capacity is calculated as:

$P_{ult} = kT \times \text{Torque rating}$

The torque rating can be determined during installation or from manufacturer guidelines.

Performance tests should always be carried out to verify the estimated geotechnical capacity.

OK Cancel

Figure 3.5.2: Helical anchor sections dialog-Geotechnical tab.

Chapter 4: Modifying Models & Viewing Results

4.1 Adding Pile Loads

Line loads in HelixPile can be added for each construction stage. The user has to click on button



in the General tab of HelixPile to add or edit the loads

Example: Adding Loads



Step 1: Click on the button (Figure 4.1.1)

Step 2: Choose to add new load (Figure 4.1.2)

Step 3: Modify the load type and magnitude (Figure 4.1.3)

Step 4: Choose whether to apply this load to all stages, or to modify it for each stage (Figure 4.1.4)

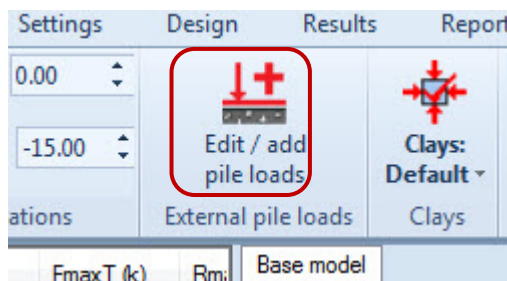


Figure 4.1.1: Add a load tool (step 1).

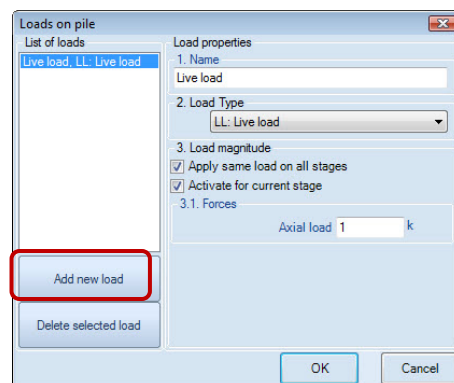


Figure 4.1.2: Add a new load (step 2).

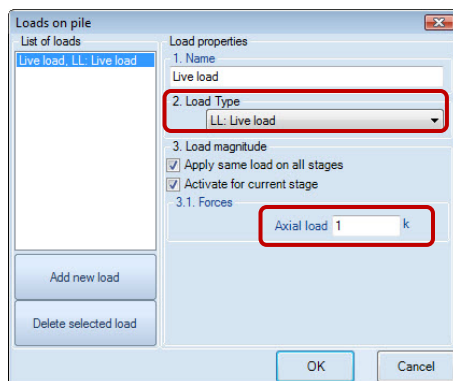


Figure 4.1.3: Define load (step 3).

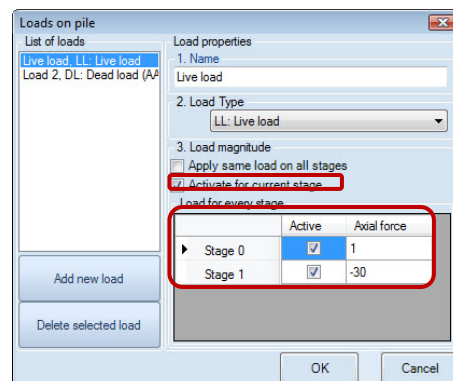


Figure 4.1.4: Change load for each stage (if needed)

4.2 Viewing Results on Main Form

Once a project is analyzed, results can be viewed on screen by selecting one or more of the options provided below:

- Show the structural ratios on screen.
- Show the critical condition results on screen
- Show the cylinder failure results on screen
- Show the individual plate results on screen
- Show the results for tension condition on screen

HelixPile can graphically represent results for all analyzed design sections and stages. The following figures show some typical on screen output diagrams and results. Output results can be visible only if the given problem has been analyzed. Feel free to explore the functionality of these toolbars.

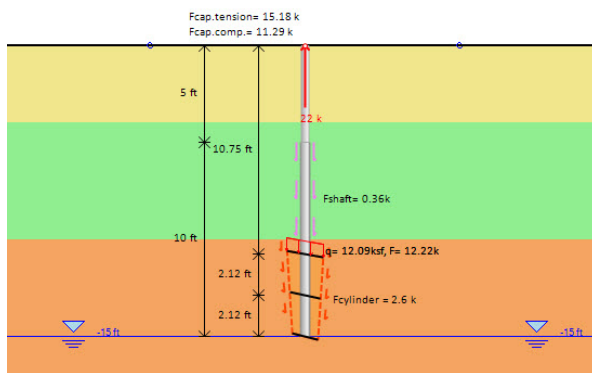


Figure 4.2.1: Critical condition results.

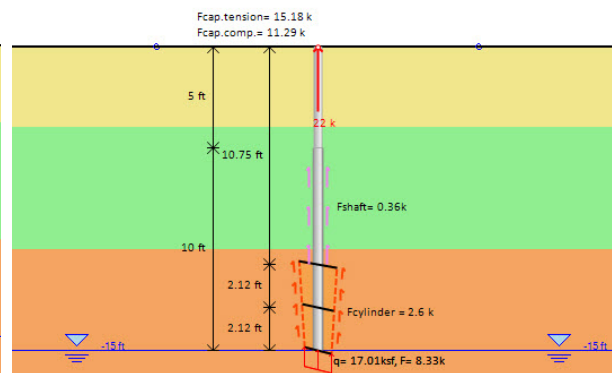


Figure 4.2.2: Cylinder failure results.

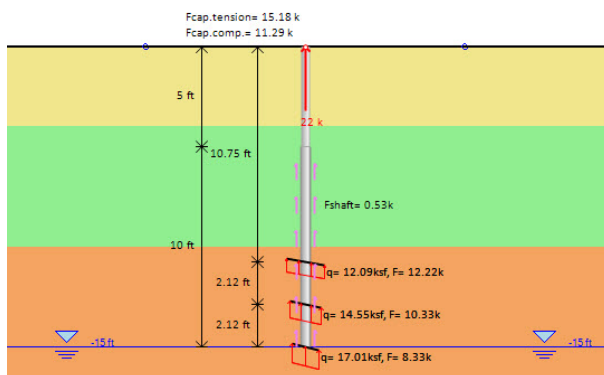


Figure 4.2.3: Individual plate method.

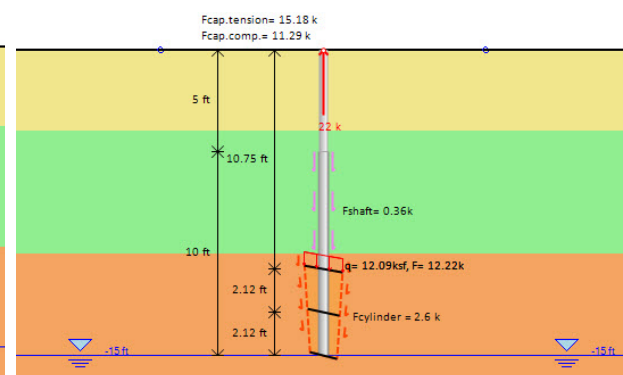


Figure 4.2.4: Tension condition.

4.3 Report Options (Printed Reports)

Once a project is analyzed, full analysis reports can be generated by selecting the Reports – Options option at the Report tab. By selecting this, we can modify the included output sections. On the left side of the dialog, we can select which design sections and stages will be included in the current report (Figure 4.3.1). From the Available Report Sections area, we can select the results and options that shall be included (Figure 4.3.2). We can also drag and drop these items at the Report Format area (Figure 4.3.3). Next, the user can select to see a preview of the report and export it in a word or PDF format at the area indicate in Figure 4.3.3.

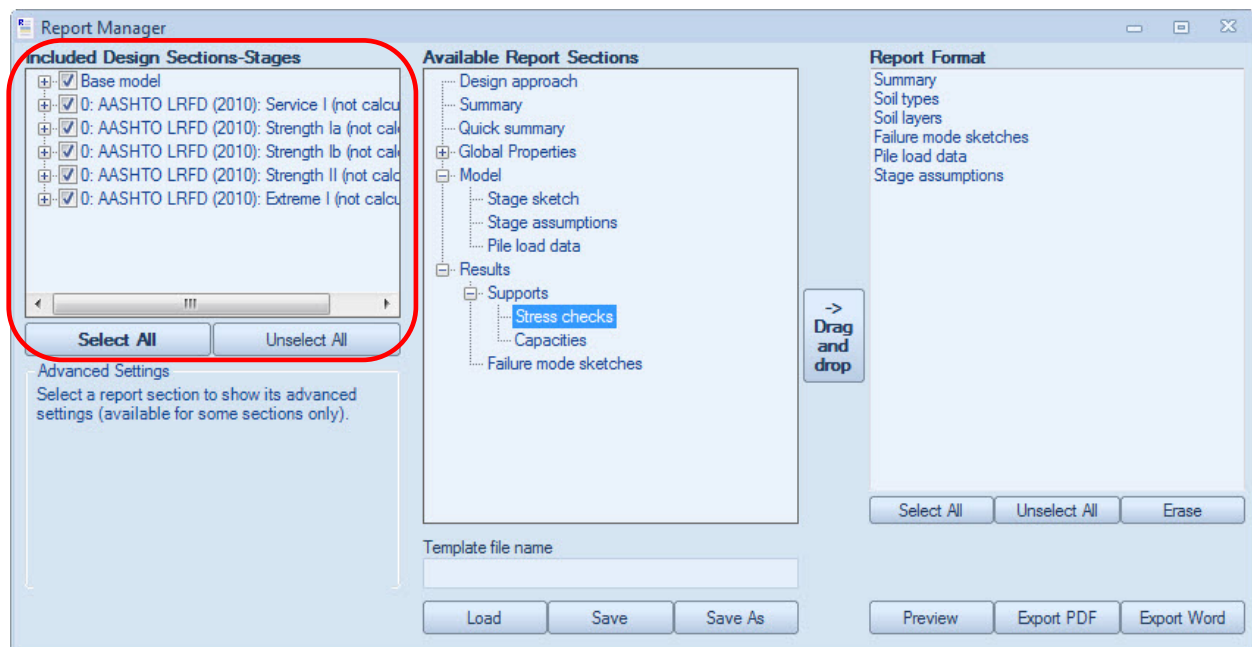


Figure 4.3.1: The included Design Sections area.

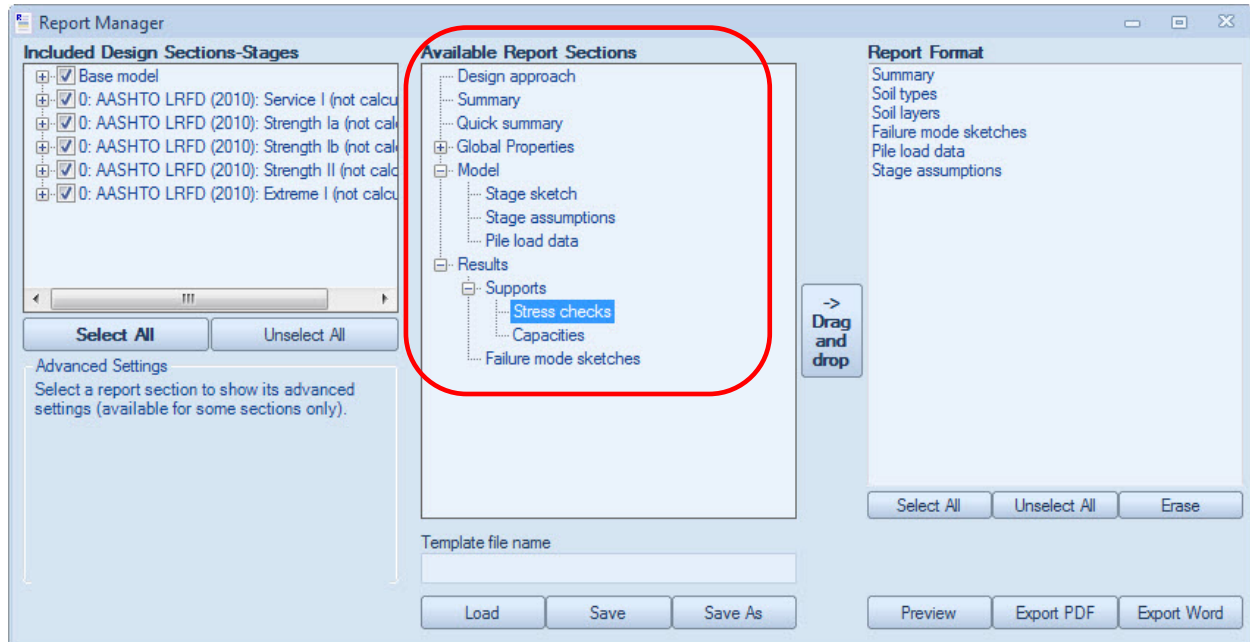


Figure 4.3.2: The Available Report Sections area.

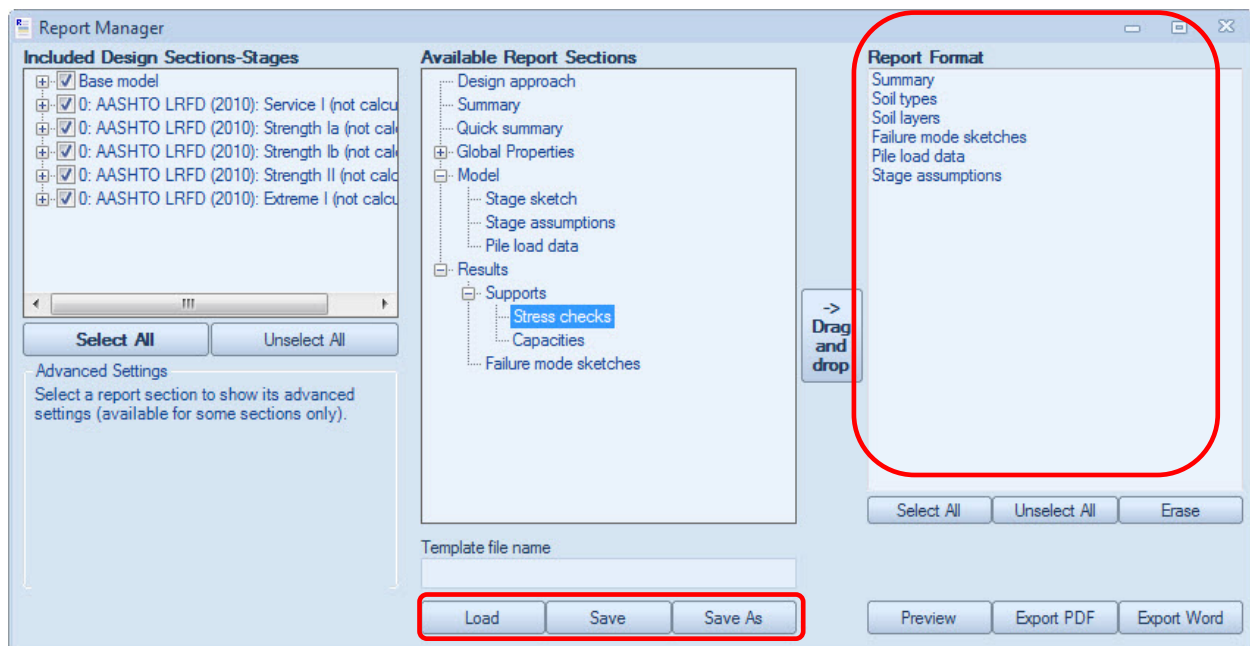
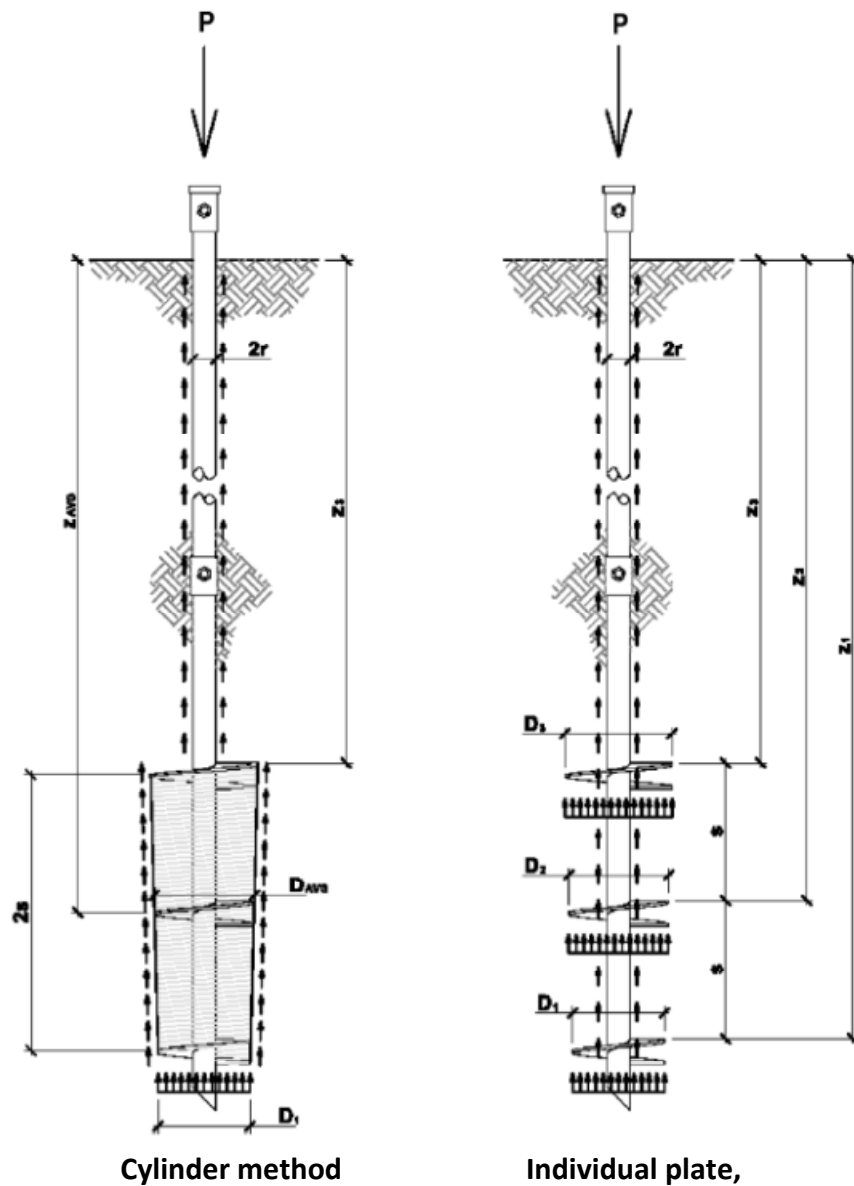


Figure 4.3.3: The Report Format area and the preview and export buttons.

Chapter 5: Theoretical background

5.1 Theoretical background

Helical piles derive their capacity from bearing and side resistance. In general, two geotechnical modes are recognized for helical pile failure: a) Individual plate failure mode, and b) cylinder failure mode, as illustrated in the following figure. If the helix spacing is large enough, then each helix will act independently and the individual bearing capacity failure will control at each plate (provided that the plates has sufficient structural capacity). On the other hand, if the helical plates are spaced close enough then the capacity will be controlled by the bottom plate bearing failure and side resistance along the cylinder bound by the helical plates (for compression).



Cylinder method Individual plate,
Figure: Helical pile theoretical failure modes.

The general bearing capacity equation used within the software is:

$$q_{ult} = c N_c + (N_q - 1) \sigma' + 0.5 \gamma D N_\gamma$$

Where:

c = Effective cohesion or undrained shear strength

D = Helical plate diameter

γ = Soil unit weight

σ' = Effective vertical stress

The bearing capacity factor N_q according to Vesic 1974 is calculated as

$$N_q = 0.5 (12 \varphi)^{\varphi/54}$$

The bearing capacity factor N_γ according to Vesic 1974 is calculated as

$$N_\gamma = (N_q - 1) \tan (1.4 \varphi)$$

Where φ is the effective friction angle in degrees.

For fine-grain soil where $\varphi = 0$ degrees, Hansen and Vesic equations yield a N_c equal to 10. However, Skempton (1951) showed both theoretically and experimentally that N_c approaches a constant value of 9 for deep foundations. Most practitioners used Skempton's result for the $\varphi = 0$ degrees condition. Under this condition, the second and third terms in bearing capacity equation go to zero because $N_q = 1$ and $N_\gamma = 0$. For this reason, the program assumes a conservative value of $N_c = 9$ even when the friction angle is greater than 0.

However, use of general bearing capacity equation would result in the calculated ultimate bearing pressure increasing without bound as q increases steadily with depth. In many cases this leads to an overprediction of bearing capacity. It has been proposed that the bearing pressure at the base of a deep foundation reaches a maximum limit at some critical depth (Meyerhof, 1951, 1976). The critical depth has been established for straight shaft piles based on a number of load tests. However, previously published critical depths for other types of deep foundations may not apply to helical piles.

Perko (2009) concluded that in summary, the ultimate bearing pressure for helical piles in coarse-grain soils may be computed using traditional bearing capacity theory by replacing the effective overburden stress, q' with the product of soil unit weight, γ , and two times the average helix diameter, D_{avg} . Within the analysis settings dialog, one can select the Use Bearing Limit option as $q = f \times \gamma (N_q - 1)$. Here the default $f=2$ value is proposed, but this can be changed according to user preference.

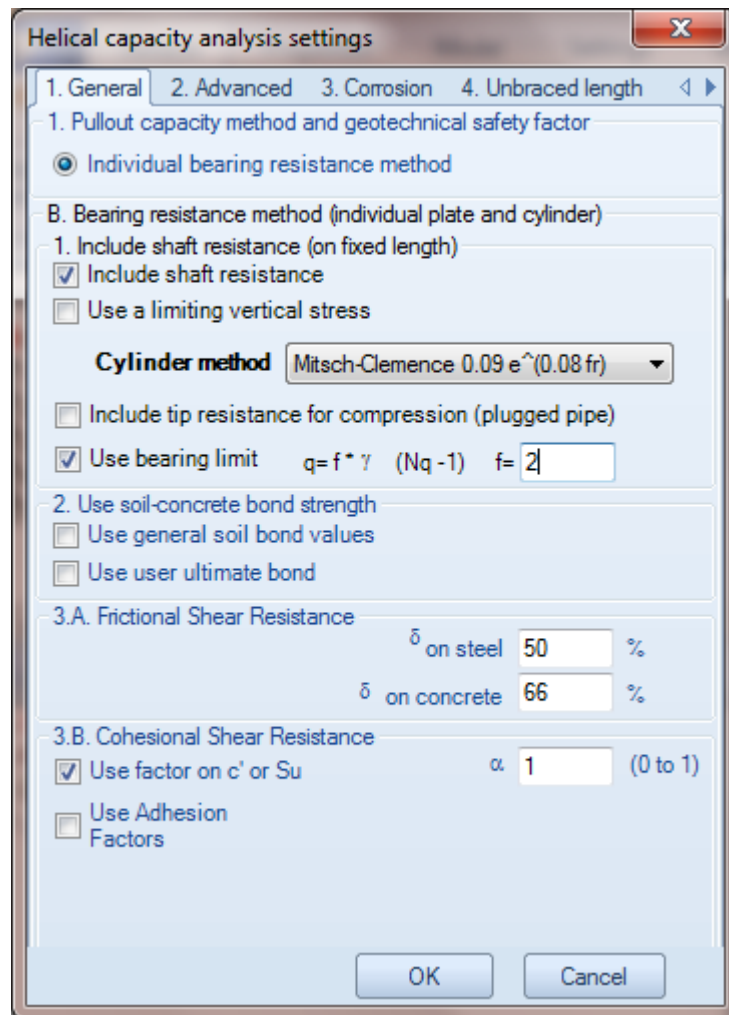


Figure 5.1: Analysis settings for limiting bearing pressure

5.2: Shaft side resistance

While in general shaft side resistance is ignored, in some occasions it might be desirable to incorporate shaft resistance within the calculations. In HelixPile there are two methods of calculating shaft resistance a) Effective stress approach, and b) soil bond values. If the option Use general soil bond values is not selected, then the program will use an effective stress approach for calculating shaft side resistance.

Figure 5.2: Shaft resistance options in analysis settings dialog

a) General soil bond values: Side resistance is calculated from the bond resistance values in the soils type dialog. This condition would be more appropriate for grouted shafts or pressure grouted shafts where the external shaft is encased in concrete.

b) Effective stress approach: In the effective stress approach, the program calculates the average effective vertical and lateral stress along the shaft. Shaft resistance is then determined from:

$$\tau = \tan(\delta \phi) \sigma'_{ave} + \alpha m c'$$

Where:

δ = Ratio of shaft to soil friction. Default δ_{steel} value is used. If the helical pile is grouted, program will use $\delta_{concrete}$. Please note that the initial percentages are general estimates and that they should be adjusted if site conditions differ.

ϕ = Effective soil friction angle

σ'_{ave} = Average normal soil stress along the shaft

α = Overall adhesion factor for cohesive component of side stress

c' = Effective cohesion or undrained shear strength for clays in undrained state.

m = Optional factor applied on cohesive side stress that reduces adhesion with tri-linear approach. In figure 5.2, for $c \leq 1$ ksf then $m = 0.8$, for $c \geq 2$ ksf $m = 0.5$ while the program performs a linear interpolation for intermediate values. The initially assumed limits are obtained from experience and general references, but should be adjusted if soil-adhesion behavior differs.

5.3 Cylinder strength method

HelixPile also examines cylinder strength to determine which axial loading condition is more critical. The program subdivides the space between plates into a number of nodes where the side shear strength on the cylinder is integrated from a side resistance of:

$$\tau = \tan(\phi) \sigma'_{ave} + c'$$

Where:

δ = Ratio of shaft to soil friction. Default δ_{steel} value is used. If the helical pile is grouted, program will use $\delta_{concrete}$. Please note that the initial percentages are general estimates and that they should be adjusted if site conditions differ.

ϕ = Effective soil friction angle

σ'_{ave} = Average normal soil stress along the shaft

If the plate sizes are different, then the program calculates and includes both the cylinder angle (from the pile axis) as well as the effective diameter along the virtual cylinder. The angle inclination of the cylinder in respect to the pile axis should make little difference in most cases.

5.4 Structural capacity calculations

Initially the design structural capacity of the pile is calculated as:

$$P_{des} = \alpha \cdot P_y$$

Where: P_{des} = Design axial capacity

P_y = Yield strength of shaft

α = Design stress factor (for allowable design typically taken as 0.5)

α is controlled from the Analysis settings dialog, tab B. Its value is automatically updated when a new structural code is selected from the Design tab.

The program also considers the structural capacity according to the selected structural code standards. This affects the compressive structural capacity when buckling is considered. HelixPile determines which loading condition is controlling in each stage, and reports the respective structural capacity.

The unbraced length below the surface has to be defined by the user depending on soil conditions for each helical pile. The program subsequently tries to determine if the pile sticks

out of the ground and incorporate this length into the effective unbraced height. Last, the effective unbraced length is calculated by multiplying by the unbraced length factor k which accounts for the end conditions of the beam. The initially assumed value is assumed as 1 (for a pinned beam at both ends), while typical values can range from 0.65 to 2 depending on the assumed fixity conditions.

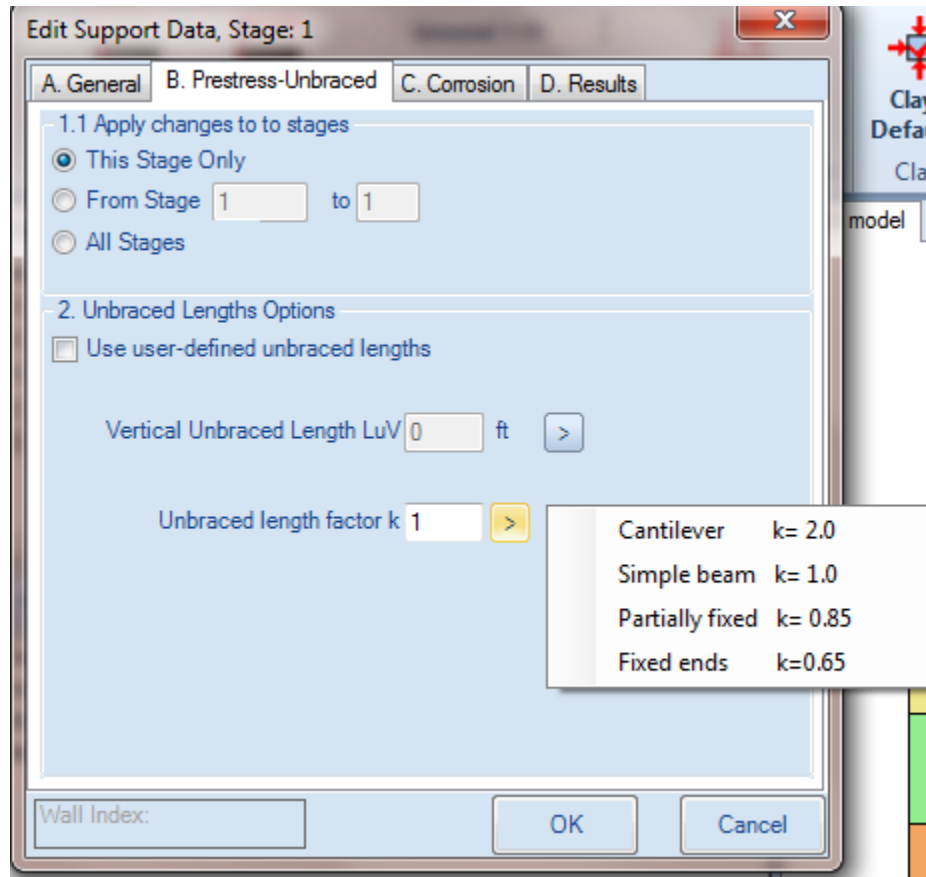


Figure 5.3: Unbraced length factor with standard recommendations

When ultimate structural codes such as AISC LRFD editions are employed, then a designer might have to consider additional safety factors applied to the structural analysis. This will also depend on whether external pile loads are factored or not. For example, if external pile loads are not factored and AISC LRFD is used, then it might be prudent to use a safety factor of 1.6 or greater to factor services loads for the buckling structural analysis according to building code standards. These settings, are initially automatically set when a building code is selected but can be adjusted from the design tab as shown in Figure 5.4. When an overall safety factor is used, the assumptions table will also show that a safety factor is applied by the name of the structural code.

☒ Adjust ultimate STR capacity by FS
Safety Factor
Structural factors

Steel Code: $\phi = 0.585$	LRFD 3rd Edition 2003 / FS= 1.6
Unbraced length	N/A +, $k = 1$

Figure 5.4: Using a safety factor on loads for ultimate structural codes such as AISC LRFD

References

- AASHTO 2010. *LRFD Bridge Design Specifications*., Washington, DC: American Association of State Highway Transportation Officials.
- A.B. Chance Co. 1992. *Anchor Corrosion Reference and Examples*, Bulletin 01-9204. Centralia, MO: A. B. Chance Co.
- 1993a. *Helical-Pier Foundation System*. Manufacturer technical support document. Centralia, MO: A.B. Chance.
- 1993b. *Tension Anchor System for Tieback Applications*. Manufacturer technical support document. Centralia, MO: A.B. Chance.
1995. *Sample Calculations for Helical Pier Application*. Manufacturer technical support document. Centralia, MO: A.B. Chance.
1996. *Buckling of Helical Anchors in Underpinning Applications*. Bulletin 01-9602, manufacturer technical support document. Centralia, MO: A.B. Chance.
2003. *Helical Screw Foundation Design Manual for New Construction*. Manufacturer technical support document. A.B. Chance: Centralia, MO
2006. *Corrosion—An Overview*. Version 1. 0, manufacturer technical support document. Centralia, MO: A. B. Chance.
- ACI318. 2008. *Building Code Requirements for Structural Concrete and Commentary*. ACI Standard, ACI Committee 318. Farmington Hills, MI: American Concrete Institute.
- Adams, J.I. and D.C. Hayes. 1967. “The Uplift Capacity of Shallow Foundations.” *Ontario Hydro Research Quarterly*, Vol. 19, No. 1, pp. 1–13.
- Adams, J.I. and T. W. Klym. 1972. “A Study of Anchors for Transmission Tower Foundations.” *Canadian Geotechnical Journal*, Vol. 9, No. 1, pp. 89–104.
- American Galvanizers Association (AGA). 2000a. *Hot-Dip Galvanizing for Corrosion Protection of Steel Products*. Englewood, CO: American Galvanizers Association.
- 2000b. *Zinc Coatings*. Englewood, CO: American Galvanizers Association.
- AISC. 1989. *Manual of Steel Construction* allowable stress design 9th edition. Chicago, IL: American Institute of Steel Construction.
- AISC. 2001. *Manual of Steel Construction* 3rd ed. Chicago, IL: American Institute of Steel Construction.
- American Society of Civil Engineers (ASCE). 2006. *Minimum Design Loads for Buildings and Other Structures (ASCE7)*. Reston, VA: American Society of Civil Engineers.
- ASTM International. 2008. “ASTM A123 Standard Specifications for Zinc (Hot-Dip Galvanized) Coatings on Iron and Steel Products.” *Annual Book of Standards*.
- West Conshohocken, PA: ASTM International.
- “ASTM A153 Standard Specifications for Zinc (Hot-Dip Galvanized) Coatings on Iron and Steel Hardware.” *Annual Book of Standards*. West Conshohocken, PA: ASTM International.
- “ASTM B633 Standard Specifications for Electrodeposited Coatings of Zinc on Iron and Steel.” *Annual Book of Standards*. West Conshohocken, PA: ASTM International.
- “ASTM A653 Standard Specifications for Continuous Sheet Galvanizing.” *Annual Book of Standards*. West Conshohocken, PA: ASTM International.
- “ASTM B695 Standard Specifications for Coatings of Zinc Mechanically Deposited on Iron and Steel.” *Annual Book of Standards*. West Conshohocken, PA: ASTM International.

- "ASTM D1143 Standard Test Method for Piles under Static Axial Compressive Load." *Annual Book of Standards*. West Conshohocken, PA: ASTM International.
- "ASTM D1586 Standard Test Method for Standard Penetration Test (SPT) and Split-Barrel Sampling of Soils." *Annual Book of Standards*. West Conshohocken, PA: ASTM International.
- "ASTM D2166 Standard Test Method for Unconfined Compressive Strength of Cohesive Soil." *Annual Book of Standards*. West Conshohocken, PA: ASTM International.
- "ASTM D2487 Standard Classification of Soils for Engineering Purposes (Unified Soil Classification System)." *Annual Book of Standards*. West Conshohocken, PA: ASTM International.
- "ASTM D2488 Standard Practice for Description and Identification of Soils (Visual-manual Procedure)." *Annual Book of Standards*. West Conshohocken, PA: ASTM International.
- "ASTM D3441 Standard Test Method for Mechanical Cone Penetration Tests of Soil." *Annual Book of Standards*. West Conshohocken, PA: ASTM International.
- "ASTM D3689 Standard Test Method for Individual Piles under Static Axial Tensile Load." *Annual Book of Standards*. West Conshohocken, PA: ASTM International.
- Atlas Systems, Inc. 2000. *Technical Guide*, ed. 1.4. Centralia, MO.
- Bassett, R.H. 1978. "Underreamed Ground Anchors." *Bulletin of Engineering Geology and the Environment*, Vol. 18, No. 1, December Springer, Berlin/Heidelberg, pp. 11–17.
- Beer, F.P. and E.R. Johnston, Jr. 1981. *Mechanics of Materials*. New York: McGraw-Hill.
- Black, D.R. and J. S. Pack. 2001. "Design and Performance of Helical Screw Piles in Collapsible and Expansive Soils in Arid Regions." In *Proceedings of the 36th Symposium, Engineering Geology and Geotechnical Engineering*, University of Nevada, Las Vegas, pp. 567–576.
2002. "Design and Performance of Helical Screw Piles in Collapsible and Expansive Soils in Arid Regions of the United States." *Proceedings of the 9th International Conference on Piling and Deep Foundations*, Nice, France. Presses Ponts et Chaussées, Paris, pp. 469–476.
- Bobbitt, B.E. and S.P. Clemence. 1987. "Helical Anchors: Applications and Design Criteria." *Proceedings of the 9th Southwest Asian Geotechnical Conference*, Bangkok, Thailand, Vol. 2, pp. 105–120.
- Bradka, T.D. 1997. *Vertical Capacity of Helical Screw Anchor Piles*, M.S. report, Department of Civil Engineering, University of Alberta, Edmonton, Alberta.
- Brinch-Hansen, J. 1961. "The Ultimate Resistance of Rigid Piles against Transversal Forces." *Danish Geotechnical Institute Bulletin No. 12*, pp. 5–9.
1963. "Discussion on Hyperbolic Stress-Strain Response: Cohesive Soils." *Journal of the Soil Mechanics and Foundation Division*, Vol. 89, pp. 241–242.
1970. "A Revised and Extended Formula for Bearing Capacity." *Danish Geotechnical Institute Bulletin No. 28*, pp. 5–11.
- Broms, B.B. 1964a. "Lateral Resistance of Piles in Cohesive Soils." *Journal of the Soil Mechanics and Foundation Division*, Vol. 90, pp. 123–156.
- 1964b. "Lateral Resistance of Piles in Cohesionless Soils." *Journal of the Soil Mechanics and Foundation Division*, Vol. 90, pp. 27–63.

- Bustamante, M. and L. Gianceselli. 1982. "Pile Bearing Capacity Prediction by Means of Static Penetrometer CPT." *Proceedings of the 2nd European Symposium on Penetration Testing*, EWOPT-II, Vol. 2, pp. 493–500.
- Carville, C.A. and R.W. Walton. 1994. "Design Guidelines for Screw Anchors." In *Proceedings of the International Conference on Design and Construction of Deep Foundations*, Orlando, FL, Vol. 2, pp. 646–655.
- . 1995. "Foundation Repair Using Helical Screw Anchors." In *Foundation Upgrading and Repair for Infrastructure Improvement* pp. 56–75. Reston, VA: American Society of Civil Engineers.
- Cerato, A.B. 2007. "Dynamically Loaded Helical Anchors for Small Wind Tower Guyed Cable Foundations." Presented at the Proceedings of the Helical Foundations and Tie-Backs Seminar, Deep Foundation Institute, New Orleans.
- Cerato, A.B. and R. Victor. 2008. "Effects of Helical Anchor Geometry on Long-Term Performance for Small Wind Tower Foundations Subject to Dynamic Loads." *Journal of Deep Foundations Institute*, Vol 2, pp 30–41.
- . "Effects of Long-Term Dynamic Loading and Fluctuating Water Table on Helical Anchor Performance for Small Wind Tower Foundations." *Journal of Performance of Constructed Facilities*. Chapel, T.A. 1998. "Field Investigation of Helical and Concrete Piers in Expansive Soils." Masters thesis, Colorado State University, Fort Collins, CO.
- Chapel, T.A. and J.D. Nelson. 2002. "Field Investigation of Helical and Concrete Piers in Expansive Soils." In *Proceedings of Geotechnical Engineering Conference*, Rio de Janeiro, Brazil.
- Chen, F.H. 1988. *Foundations on Expansive Soils (Developments in Geotechnical Engineering)*. Amsterdam: Elsevier Science Publishers.
- Chuan, H.S. 2006. "Uplift Capacity of Helical Anchor in Sand." undergraduate thesis, University of Technology, Malaysia.
- Clemence, S.P. 1985. "Uplift Behavior of Anchor Foundations in Soil." In *Proceedings of a Session Sponsored by the Geotechnical Engineering Division of ASCE*, Detroit, MI.
- Clemence, S.P., L.K. Crouch, and R.W. Stephenson. 1994. "Uplift Capacity of Helical Anchors in Soils." *Proceedings of the 2nd Geotechnical Engineering Conference*, Cairo, Egypt, Vol. 1, pp. 332–343.
- Clemence, S.P. and Y. Li. 2008. "Review of Helical Foundations over Twenty Years." Presented at the Proceedings of Helical Foundations and Tie-Backs Specialty Seminar, Deep Foundation Institute, Los Angeles, CA.
- Clemence, S.P., and F.D. Pepe Jr. 1984. "Measurement of Lateral Stress around Multihelix Anchors in Sand." *Geotechnical Testing Journal*, Vol. 7, No. 3, pp. 145–152.
- Clemence, S.P., and A. P. Smithling. 1984. "Dynamic Uplift Capacity of Helical Anchors in Sand." In *Proceedings of the 4th Australia-New Zealand Conference, Geomechanics*, No. 1, pp. 88–93.
- Clemence, S.P. and C.J. Veesaert. 1977. "Dynamic Pullout Resistance of Anchors in Sand." *Proceedings of the International Symposium on Soil-Structure Interaction*, Roorkee, India, pp. 389–397.

- Cole, W.H. 1978. "An Innovative Use for Multi-Helix Anchors." Presented at the EEI T&D Subcommittee, Key Biscayne, FL (unpublished).
- Craig, B.D. 1995. *Handbook of Corrosion Data*. Materials Park, OH: ASM International.
- Curle, R. 1995. "Screw Anchors Economically Control Pipeline Buoyancy in Muskeg." *Oil and Gas Journal*, Vol. 93, No. 17, pp. 49–54.
- Dai, S.H. and M.O. Wang. 1992. *Reliability Analysis in Engineering Applications*. New York: Van Nostrand, Reinhold.
- Das, B.M. 1990. *Principles of Geotechnical Engineering* 2nd ed. Boston: PWS-Kent Publishing Company.
- Davisson, M.T. 1972. "High Capacity Piles." In *Proceedings of Lecture Series on Innovations in Foundation Construction*, ASCE, Illinois Section, Chicago (March), pp. 81–112.
- Davisson, M.T. and H.L. Gill. 1963. "Laterally Loaded Piles in Layered Soil System." *Journal of Soil Mechanics and Foundation Division*, ASCE, Vol. 89, No. SM3, pp. 63–94.
- Deardorff, D. 2007. "Torque Correlation Factors for Round Shaft Helical Piles." In *Proceedings of the 32nd Annual Conference of the Deep Foundation Institute*, Colorado Springs, CO, pp. 439–450.
- DeBeer, E.E. 1967/1968. "Proefondervindlijke Bijdrage Tot de Studie van Het Grensdrag Vermogen van Zand onder Funderingen op Staal." *Tijdschrift der Openbar Verken van België*, No. 6 (1967), and Nos. 4, 5, and 6 (1968).
1970. "Experimental Determination of Shape Factor and Bearing Capacity Factors of Sand." *Geotechnique*, Vol. 20, No. 4, pp. 387–411.
- Diewald, G.A. 2003. *A Modified Soil Suction Heave Prediction Protocol: With New Data from Denver Area Expansive Soil Sites*, MS Thesis, University of Colorado at Denver, Denver, CO.
- Dobry, R., E. Vicente, M.J. O'Rourke, and J.M. Roesset. 1982. "Horizontal Stiffness and Damping of Single Piles." *Journal of the Geotechnical Engineering Division*, Vol. 108, No. GT3, pp. 439–459.
- Downey, S. 2008. "A Quality Foundation." Presented at the Proceedings of Helical Foundations and Tie-Backs Specialty Seminar (November) Deep Foundation Institute, Los Angeles, CA.
- Duncan, J.M. 2000. "Factors of Safety and Reliability in Geotechnical Engineering." *Journal of Geotechnical and Geoenvironmental Engineering*, Vol. 126, No. 4, pp. 307–316.
- Duncan, J.M. and S.G. Wright. 2005. *Soil Strength and Slope Stability*. New York: John Wiley and Sons.
- Duzceer, R. and A. Saglamer. 2002. "Evaluation of Pile Load Test Results." *Proceedings of the 9th International Conference on Piling and Deep Foundations* Nice, France. Paris: Presses Ponts et Chaussees, pp. 469–476.
- El Marsafawi, H., Y.C. Han, and M. Novak. 1992. "Dynamic Experiments on Two Pile Groups." *Journal of Geotechnical Engineering*, Vol. 118, No. 4, pp. 576–592.
- Elias, V. 2000. "Corrosion/Degradation of Soil Reinforcement for Mechanically Stabilized Earth Walls and Reinforced Soil Slopes." Report No. FHWA-NHI-00-044, Federal Highway Administration.
- Fellenius, B.H. 2001a. "We Have Determined the Capacity, then What?" *Fulcrum* (Deep Foundation Institute) (Fall), pp. 23–26.

- 2001b. "What Capacity Value to Choose from the Results of a Static Load Test." *Fulcrum* (Deep Foundation Institute) (Winter), pp. 19–22.
- Fleming, W.G.K., A.J. Weltman, M.F. Randolph, and W.K. Elson. 1985. *Piling Engineering*. Glasgow: Surrey University Press.
- Gazetas, G. and R. Dobry. 1984a. "Horizontal Response of Piles in Layered Soils." *Journal of Geotechnical Engineering*, Vol. 110, No. 1, pp. 20–40.
- 1984b. "Simple Radiation Damping Model for Piles and Footings." *Journal of Engineering Mechanics*, Vol. 110, No. 6, pp. 937–956.
- Ghaly, A.M. and S.P. Clemence. 1998. "Pullout Performance of Inclined Helical Screw Anchors in Sand." *Journal of Geotechnical and Geoenvironmental Engineering*, Vol. 124, No. 7, pp. 617–627.
1999. "Closure to Pullout Performance of Inclined Helical Screw Anchors in Sand." *Journal of Geotechnical and Geoenvironmental Engineering*, Vol. 125, No. 12, pp. 1102–1104.
- Ghaly, A.M. and A.M. Hanna. 1991a. "Experimental and Theoretical Studies on Installation Torque of Screw Anchors." *Canadian Geotechnical Journal*, 28, No. 3, pp. 353–364.
- 1991b. "Stress Development in Sand Due to Installation and Uplifting of Screw Anchors." *Proceedings of the 4th International Conference on Piling and Deep Foundations*, Vol. 1, pp. 565–570.
- Ghaly, A. and A. Hanna. 1992. "Stresses and Strains around Helical Screw Anchors in Sand." *Soils and Foundations*, Vol. 32, No. 4, pp. 27–42.
1993. "Model Investigation of the Performance of Single Anchors and Groups of Anchors." *Canadian Geotechnical Journal*, Vol. 31, pp. 273–284.
- Ghaly, A.M. and A.M. Hanna. 1994a. "Model Investigation of the Performance of Single Anchors and Groups of Anchors." *Canadian Geotechnical Journal*, Vol. 31, pp. 273–284.
- 1994b. "Ultimate Pullout Resistance of Single Vertical Anchors." *Canadian Geotechnical Journal*, Vol. 31, No. 5, pp. 661–672.
- Ghaly, A., A. Hanna, and M. Hanna. 1991a. "Uplift Behavior of Screw Anchors in Sand. I: Dry Sand." *Journal of Geotechnical Engineering*, Vol. 117, No. 5, pp. 773–793.
- 1991b. "Uplift Behavior of Screw Anchors in Sand. II: Hydrostatic and Flow Conditions." *Journal of Geotechnical Engineering*, Vol. 117, No. 5, pp. 794–808.
- 1991c. "Installation Torque of Screw Anchors in Dry Sand." *Soils and Foundations, Japanese Society of Soil Mechanics and Foundation Engineering*, Vol. 31, No. 2, pp. 77–92.
- Ghaly, A.M., Hanna, A., Ranjan, G. and Hanna, M. 1991. "Helical Anchors in Dry and Submerged Sand Subjected to Surcharge." *Journal of Geotechnical Engineering*, Vol. 117, No. 10, pp. 1463–1470.
- Gill, H.S. and J.J. Udvari. 1980. "Pullout Tests: Multi-Helix Screw Anchors." report prepared for the Virginia Electric and Power Company, Richmond, VA.
- Gregory, S. and R.M. Hoyt. 2005. "Strength Assessment of Helix Plates for Helical Piles." *Proceedings of the Helical Foundations and Tiebacks Seminar*. Torrance, CA: Deep Foundation Institute.
- Hanna, A. and A. Ghaly. 1992. "Effects of K_0 and Overconsolidation on Uplift Capacity." *Journal of Geotechnical Engineering*, Vol. 118, No. 9, pp. 1449–1469.

1994. "Ultimate Pullout Resistance of Groups of Vertical Anchors." *Canadian Geotechnical Journal*, Vol. 31, No. 5, pp. 673–682.
- Hanna, T.H., E. Sivapalan, and A. Senturk. 1978. "The Behavior of Dead Anchors Subjected to Repeated and Alternating Loads," *Ground Engineering*, Vol. 2, No.4, pp. 28–34.
- Hardesty, R. 2007. "Helical Piers vs. Drilled Concrete Piers in Highly Expansive Soil Areas." Hardesty Consulting, LLC, Denver, CO, www.helicalpierworld.com.
- Hargrave, R.L. and R.E. Thorsten. 1992. "Helical Piers in Expansive Soils of Dallas, Texas." *7th International Conference on Expansive Soils*, Vol. 1, pp. 125–130. Lubbock, TX: Texas Tech University Press.
- Helmers, M.J., J.M. Duncan, and G.M. Filz. 1997. "Use of Ultimate Load Theories for Design of Drilled Shaft Sound Wall Foundations." Report of the Virginia Transportation Research Council.
- Hovland H.J. 1993. "Discussion of Helical Anchors in Dry and Submerged Sand Subjected to Surcharge." *Journal of Geotechnical Engineering*, Vol. 119, No. 2, pp. 391–392.
- Hoyt, R. 2007. *Engineering Manual: Ram Jack Helix Screw Anchors, Ram Jack Steel Piers*. Ada, OK: Ram Jack Systems Distribution, LLC.
- Hoyt, R.M. and S.P. Clemence. 1989. "Uplift Capacity of Helical Anchors in Soil." *Proceedings of the 12th International Conference on Soil Mechanics and Foundation Engineering*, Rio de Janeiro, Brazil Vol. 2, pp. 1019–1022.
- Hoyt, R.M., G. Seider, L.C. Reese, M. Hon, and S. Wang. 1995. "Buckling of Helical Anchors Used for Underpinning." In *Foundation Upgrading and Repair for Infrastructure Improvement*, pp. 89–108. Reston, VA: American Society of Civil Engineers.
- Hsu, T.T.C. 1968. "Torsion of Structural Concrete-Plain Concrete Rectangular Sections." *Torsion of Structural Concrete*, Portland Cement Association, Bulletin D134, pp. 203–238.
- Huang, F.C., I. Mohmood, M. Joolazadeh, and G.W. Axten. 1995. "Design Considerations and Field Load Tests of a Helical Anchoring System for Foundation Renovation." *Foundation Upgrading and Repair for Infrastructure Improvement*, pp. 76–88. Reston, VA: American Society of Civil Engineers.
- ICC-Evaluation Services. 2003. "AC228 Acceptance Criteria for Corrosion Protection of Steel Foundation Systems Using Polymer (EAA) Coatings." www.icc-es.org.
2004. "Dixie Anchoring Helical Foundation System.", ICC-ES Legacy Report 21-47, www.icc-es.org.
2007. "AC358 Acceptance Criteria for Helical Pile Foundations and Devices." www.icc-es.org.
- Industrial Galvanizers America. 1999. "Product Galvanizing Brochure. International Business Publishers, Atlanta, GA.
- International Code Council. 2006. *International Building Code (IBC)*. Washington, DC: International Code Council.
- Johnston, G.H. and B. Ladanyi. 1974. "Field Tests of Deep Power-Installed Screw Anchors in Permafrost." *Canadian Geotechnical Journal*, Vol. 11, No. 3, pp. 348–359.
- Jones, D.A. 1986. *Principles and Prevention of Corrosion* 2nd ed. London: Prentice-Hall.
- Klym, T.W., H.S. Radhakrishna, et al. 1986. "Helical Plate Anchors for Tower Foundations." In *Proceedings of Symposium on Anchor Systems in Geotechnical Engineering*. Toronto: Canadian Geotechnical Society.

- Lutenegger, A.J. 2003. “Helical Screw Piles and Screw Anchors—An Historical Prospective and Introduction.” In *Proceedings of the Helical Foundations and Tie-Backs Seminar*, Deep Foundation Institute, Cincinnati, OH.
- Lutenegger, A.J., B.L. Smith, and M.G. Kabir. 1988. “Use of In Situ Tests to Predict Uplift Performance of Multi-Helix Anchors.” *Special Topics in Foundations*, ASCE, pp. 93–110.
- MacLean Dixie. 2009. *Product Information Bulletin No. 008-0*, Manufacturer Product Literature, MacLean Dixie, Franklin Park, IL.
- McKeen, R.G. and L.D. Johnson. 1990. “Climate-Controlled Soil Design Parameters for Mat Foundations.” *Journal of Geotechnical Engineering*, 116(7), pp. 1073–1094.
- McOmber, R.M. and R.W. Thompson. 2000. “Verification of Depth of Wetting for Potential Heave Calculations.” In *Advances in Unsaturated Geotechnics: Proc. Sessions of Geo-Denver*, C. Shackelford, S. Houston, and N.Y. Chang, eds., pp. 409–422.
- Meyerhof, G.G. 1951. “The Ultimate Bearing Capacity of Foundations.” *Geotechnique*, Vol. 2, No. 4, pp. 301–331.
1976. “Bearing Capacity and Settlement of Pile Foundations.” *Journal of Geotechnical and Geoenvironmental Engineering*, Vol. 102, No. 3, pp. 195–228.
- Meyerhof, G.G. and J.I. Adams. 1968. “The Ultimate Uplift Capacity of Foundations.” *Canadian Geotechnical Journal*, Vol. 5, No. 4, pp. 224–244.
- Michaelides, O., G. Gazetas, G. Bouckovalas, and E. Chrysikou. 1997. “Approximate Non-linear Dynamic Axial Response of Piles.” *Geotechnique*, Vol. 48, No. 1, pp. 33–53.
- Miller, F.E., J.E. Foss, and D.C. Wolf. 1981. *ASTM STP 741*, American Society for Testing and Materials, p. 19.
- Mitsch, M.P. and S.P. Clemence. 1985a. “The Uplift Capacity of Helix Anchors and Sand.” *Uplift Behavior of Anchor Foundations in Soil*, ASCE, pp. 26–47.
- 1985b. “Uplift Behavior of Anchor Foundations in Soil.” *Journal of Geotechnical Engineering*, pp. 26–47.
- Mizuno, H. 1987. “Pile Damage during Earthquake in Japan (1923–1983).” *Dynamic Response of Pile Foundations—Experiment, Analysis and Observation*, ASCE Special Geotechnical Publication No. 11, T. Nogami, Ed., pp. 53–78.
- Mooney, J.S., S. Adamczak Jr., and S.P. Clemence. 1985. “Uplift Capacity of Helix Anchors in Clay and Silt.” *Uplift Behavior of Anchor Foundations in Soil*, ASCE pp. 48–72.
- Narasimha Rao, S. and Y.V.S.N. Prasad. 1993a. “Behavior of Model Screw Anchors in Soft Clays.” *Geotechnique*, Vol. 43, No. 44, pp. 605–614.
- 1993b. “Estimation of Uplift Capacity of Helical Anchors in Clays.” *Journal of Geotechnical Engineering*, Vol. 119, No. 2, pp. 352–357.
- Narasimha Rao, S., Y.V.S.N. Prasad, and C.V. Prasad. 1990. “Experimental Studies on Model Screw Pile Anchors.” *Proceedings of the Indian Geotechnical Conference, Bombay*, pp. 465–468.
- Narasimha Rao, S., Y.V.S.N. Prasad, and M.D. Shetty. 1991. “The Behavior of Model Screw Piles in Cohesive Soils.” *Soil and Foundations*, Vol. 31, No. 2, pp. 35–50.
- Narasimha Rao, S., Y.V.S.N. Prasad, M.D. Shetty, and V.V. Joshi. 1989. “Uplift Capacity of Screw Pile Anchors.” *Geotechnical Engineering*, Vol. 20, No. 2, pp. 139–159.
- Narasimha Rao, S., Y.V.S.N. Prasad, and C. Veeresh. 1993. “Behavior of Embedded Model Screw Anchors in Soft Clays.” *Geotechnique*, Vol. 43, pp. 605–614.

- Nasr, M.H. 2004. “Large Capacity Screw Piles.” In *Proceedings of International Conference: Future Vision and Challenges for Urban Development*, Cairo, Egypt.
- Nasr, M.H. 2007. “Use of Large Capacity Screw Piles—High Pressure Grouted Piles—Barbados.” *Proceedings of the Helical Foundations and Tie-Backs Seminar*, Deep Foundation Institute, New Orleans.
- Nasr, M.H. 2008. “Behavior of High Pressure Grouted Screw Piles in Santa Clara, CA.” www.helicalpiersystems.com/pdf/HPS-Paper%20Santa%Clara.pdf.
- Novak, M., and F. Aboul-Ella. 1978. “Stiffness and Damping of Piles in Layered Media.” *Proceedings of the ASCE Geotechnical Engineering Division Specialty Conference on Earthquake Engineering and Soil Dynamics*, June 19-21 1978, Pasadena, CA, Vol. 2, pp. 704–719. Reston, VA: ASCE Press.
- Novak, M., and B. El Sharnouby. 1983. “Stiffness Constants for Single Piles.” *Journal of Geotechnical Engineering*, ASCE, Vol. 109, No. 7, pp. 961–974.
- National Park Service (NPS). 2007. *List of National Historic Landmarks*. www.nps.gov. U.S. Department of the Interior.
- Occupational Health and Safety Administration (OSHA). 2008. *Excavations* (Document 2226). Washington, DC: Occupational Safety and Health Administration.
- Pack, J.S. 2000. “Design of Helical Piles for Heavily Loaded Structures.” In *New Technological and Design Developments in Deep Foundations*, Reston, VA: ASCE Press, pp. 353–367.
2004. *Practical Design and Inspection Guide for Helical Screw Piles and Helical Tension Anchors*, 3rd ed. Aurora, CO: IMR, Inc.
2006. “Performance of Square Shaft Helical Pier Foundations in Swelling Soil.” *Proceedings of Geo-volution*, ASCE and AGU Joint Conference Denver, CO ASCE Geotechnical Practice Publication No. 4, pp. 76–85.
2007. “Design, Specification and Installation of Square Shaft Helical Piers in Expansive Soils.” *Proceedings of the 32nd Annual Conference of the Deep Foundation Institute*, Colorado Springs, CO, pp. 321–330.
- Pack, J.S. and K.M. McNeill. 2003. “Square Shaft Helical Screw Piles in Expansive Clay Areas.” *Proceedings of the 12th Pan American Conference on Soil Mechanics and Geotechnical Engineering*, pp. 1825–1832.
- Parry, R.H.G. 1977. “Estimating Bearing Capacity of Sand from SPT Values.” *Journal of Geotechnical Engineering Division*, Vol. 103, No. GT 9, pp. 1014–1019.
- Peck, R.B. 1969. “Deep Excavations and Tunneling in Soft Ground,” *Proceedings of 7th International Congress of Soil Mechanics and Foundation Engineering, State-of-Art Volume*, pp. 225–290.
- Peck, R.B., W.E. Hanson, and T.H. Thornburn. 1965. *Foundation Engineering*, New York: John Wiley and Sons.
- Perko, H.A. 1999. “Summary of Earth Retaining Methods Utilizing Helical Anchors.” *Magnum Piering Technical Reference Guide, Engineering Analysis*, Section 3, Cincinnati, OH: Magnum Piering, Inc.
2001. “Energy Method for Predicting the Installation Torque of Helical Foundations and Anchors.” In *New Technological and Design Developments in Deep Foundations*, pp. 342–352. Reston, VA: American Society of Civil Engineers.

2003. "Lateral Capacity and Buckling of Helix Pier Foundations." In *Proceedings of the Helical Foundations and Tie-Backs Specialty Seminar*, Deep Foundation Institute, Cincinnati, OH.
- 2004a. "Introduction to Corrosion and Galvanizing of Helix Foundations." In *Proceedings of the Helical Foundations and Tie-Backs Specialty Seminar*, Deep Foundation Institute, Tampa, FL.
- 2004b *Helipost™ Engineering Handbook*, Rev. 2. Fort Collins, CO: Secure Piers, LLC.
2005. "Underpinning and Shoring for Underground MRI Research Facility at Ohio State University." In *Proceedings of Underground Construction in Urban Environments*, Specialty Seminar by ASCE Metropolitan Section Geotechnical Group at the Geo-Institute of ASCE, New York, NY.
- 2006a "Geotechnical Techniques Used in Planetary Exploration." Keynote address, *Proceedings of Geo-volution*, ASCE and AGU Joint Conference, Denver, CO, pp. 109–119.
- 2006b "ICC-ES Acceptance Criteria for Helical Foundations and Devices." In *Proceedings of the Helical Foundations and Tie-Backs Specialty Seminar*, Deep Foundation Institute, Newark, NJ.
- 2006c. "Installation Torque as a Predictor of Helical Pier Axial Capacity." Electronic publication, www.helicalpierworld.com.
- 2007a. "Lateral Resistance of Helical Foundations for Hurricane-Prone Coastal Areas." In *Proceedings of the Helical Foundations and Tie-Backs Specialty Seminar*, Deep Foundation Institute, New Orleans, LA.
- 2007b. "Evidence of Seismic Resistance of Helical Foundations." In *Proceedings of the Helical Foundations and Tie-Backs Specialty Seminar*, Deep Foundation Institute, New Orleans, LA.
- 2007c. "Creating Acceptance for Helical Foundations." Code Updates, *Structure Magazine*. December, pp. 49–50.
- 2008a "Helical Pile Foundation for Alexan Broadway Parking Structure." *Proceedings of Case Histories in Deep Foundations*, Deep Foundation Institute, Cincinnati, OH.
- 2008b "Helical Piles in the Building Codes." *Proceedings of the Helical Foundations and Tie-Backs Specialty Seminar*, Deep Foundation Institute, Los Angeles, CA.
2009. "Helical Piles: A practical guide to design and installation" John Willey & Sons
- Perko, H.A. and J.B. Boulden. 2008. "Lateral Earth Pressure on Lagging in Soldier Pile Wall Systems." *DFI Journal*, Vol. 2 (November), pp. 52–60.
- Perko, H.A., and R.A. Doner. (in press). "Full-Displacement, Augered Friction Piles and a Method for Estimating Capacity." *Proceedings of the 34th Annual Conference of the Deep Foundation Institute*, Kansas City, MO.
- Perko, H.A. and S. Rupiper. 2002. *Helical Pile Engineering Handbook*. Manufacturer technical Literature. Larkspur, CO: Precision Pier USA.
- Prasad, Y.V.S.N. and S. Rao. 1994. "Pullout Behavior of Model Piles and Helical Pile Anchors Subjected to Lateral Cyclic Loading." *Canadian Geotechnical Journal*, Vol. 31, No. 1, pp. 110–119.

- Prasad, Y.V.S.N. and S.N. Rao. 1996. "Lateral Capacity of Helical Piles in Clays." *Journal of Geotechnical Engineering*, Vol. 122, No. 11, pp. 938–941.
- Prayer, J.H., et al. 1980. Material Performance (as referenced by Jones, 1986).
- Puri, V.K., R.W. Stephenson, E. Dziedzic, and L. Goen. 1984. "Helical Anchor Piles under Lateral Loading." *Laterally Loaded Deep Foundations: Analysis and Performance*, ASTM STP 835, Langer, Mosley, and Thompson, Eds. West Conshohocken, PA: American Society for Testing and Materials, pp. 194–213.
- Radhakrishna, H.S. 1972. "Helix Anchor Tests in Stiff Fissured Clay." Ontario Hydro Research Division Report No. 72-12-H. Nanticoke, Ontario, Ontario Hydro.
- Radhakrishna, H.S. 1976. "Helix Anchor Tests in Sand." Ontario Hydro Research Division Research Report No. 76-130-K, pp. 1–33.
- Rao, S.N. and Y.V.S.N. Prasad. 1993. "Estimation of Uplift Capacity of Helical Anchors in Clays." *Journal of Geotechnical Engineering*, Vol. 119, No. 2, pp. 352–357.
- Rao, S.N., Y.V.S.N. Prasad, and C.V. Prasad. 1990. "Experimental Studies on Model Screw Pile Anchors." *Proceedings of the Indian Geotechnical Conference*, Vol. 1, Bombay, pp. 465–468.
- Rao, S.N., Y.V.S.N. Prasad, and M.D. Shetty. 1991. "Behavior of Model Screw Piles in Cohesive Soils." *Soils and Foundations*, Vol. 31, No. 2, pp. 35–50.
- Rao, S.N., Y.V.S.N. Prasad, and C. Veeresh. 1993. "Behavior of Embedded Model Screw Anchors in Soft Clays." *Geotechnique*, Vol. 43, No. 4, pp. 605–614.
- Read, A.A.L. and S. Sritharan. 1993. "Reconnaissance Report on the Ormond Earthquake—August 10, 1993." *Bulletin of the New Zealand National Society for Earthquake Engineering*, Vol. 26, No. 3, pp. 292–308.
- Revie, R.W. (ed.). 2000. *Uhlig's Corrosion Handbook*, 2nd ed. Electrochemical Society Series. New York: Wiley Interscience.
- Robinson, K.E. and H. Taylor. (1969). "Selection and Performance of Anchors for Guyed Transmission Towers." *Canadian Geotechnical Journal*, Vol. 6, pp. 119–135.
- Rodgers, T.E. Jr. 1987. "High Capacity Multi-Helix Screw Anchors for Transmission Line Foundations." *Foundation for Transmission Line Towers*, ASCE Reston, VA, ASCE Press, pp. 81–95.
- Romanoff, M.[1957] 1989. *Underground Corrosion*. National Bureau of Standards No. 579. Reprinted by NACE, Houston, TX.
1972. "NBS Papers on Underground Corrosion of Steel Piling." National Bureau of Standards Monograph 127. Gaithersburg, MD: NIST.
- Rupiper, S. 1990. "Helix Piers Are Solutions for Column Reactions." In *Abstracts of the Proceedings of the 8th Structures Congress*. Baltimore, MD: ASCE.
1994. "Helical Plate Bearing Members, A. Practical Solution to Deep Foundations." In *Proceedings of the International Conference on the Design and Construction of Deep Foundations*, Vol. 2, Hawthorne, NJ: Deep Foundation Institute, pp. 980–991.
- Rupiper, S. and W.G. Edwards. 1989. "Helical Bearing Plate Foundations for Underpinning." In *Proceedings of Foundation Engineering Congress/SCE/CO Division*, Evanston, IL, June 25–29.
- Sailer, D. and B. Soth. 2004. "Helical Pier Foundations for Problem Sites." *Journal of Light Construction* (May).

- Schmidt, R. 2004. "Screw Piles: Uses and Considerations." *Structure Magazine*(June), pp. 25–31.
- Seider, G. 1993a. "Eccentrically Loaded Helical Pier Systems," Bulletin 01-9303. Centralia, MO: A.B. Chance Company.
- 1993b. "Eccentric Loading of Helical Piers for Underpinning." In *Proceedings of the 3rd International Conference on Case Histories in Geotechnical Engineering*, St. Louis, MO, Vol. 1, pp. 139–145.
2004. "Helical Foundations: What an Engineer Needs to Know." *Structure Magazine*, Vol. 11, No. 6, pp. 27–28.
- Skempton, A.W. 1951. "The Bearing Capacity of Clays." In *Proceedings of the Building Research Congress*, Vol. 1, pp. 180–189.
- Slemons, P.E. 2008. "A Rational Approach to Calculating Torque to Capacity in Cohesive Soils." *Proceedings of the Helical Foundations and Tie-Backs Specialty Seminar*. Deep Foundation Institute, Los Angeles, CA, Hawthorne, NJ: DeepFoundation Institute.
- Stagg, K.G. and O.C. Zienkiewicz. 1968. *Rock Mechanics in Engineering Practice*, New York: John Wiley and Sons.
- Sun, K. and J.A. Pires. 1993. "Simplified Approach for Pile and Foundation Interaction Analysis." *Journal of Geotechnical Engineering*, Vol. 119 No. 9, pp. 1462–1479.
- Tabesh, A. and H.G. Poulos. 1999. "The Effect of Soil Yielding on Internal Pile Response." In *Proceedings of the 2nd International Conference on Earthquake Geotechnical Engineering*, ed. Seco e Pinto vol. 1, 327–333. Rotterdam: A.A. Balkema.
- Tappenden, K.M. 2004. "Predicting the Axial Capacity of Screw Piles Installed in Western Canadian Soils." Master's thesis, University of Alberta, Edmonton, Alberta.
- Terzaghi, K. 1943. *Theoretical Soil Mechanics*. New York: John Wiley and Sons.
- Terzaghi, K. and R.B. Peck. 1967. *Soil Mechanics in Engineering Practice*. New York: John Wiley and Sons.
- Thompson, R.W., W. Rethamel, and H.A. Perko. 2006. "Comparison of Constant Volume and Oedometer Swell Pressures." In *Proceedings of Unsat 2006*, American Society of Civil Engineers, Phoenix, AZ.
- Tomlinson, M.J. 1986. *Foundation Design and Construction*, 5th ed. New York: John Wiley and Sons.
- Trofimenkov, J.G., and L.G. Maruipolshii. 1965. "Screw Piles Used for Mast and Tower Foundations." In *Proceedings of the 6th International Conference on Soil Mechanics and Foundation Engineering*, Vol. 2, pp. 328–332.
- Udwari, J.J., T.D. Rodgers, and H. Singh. 1979. "A Rational Approach to the Design of High Capacity Multi-Helix Screw Anchors." *Proceedings of the 7th Annual IEEE/PES, Transmission and Distribution Exposition*, Piscataway, NJ: IEEE Publishing, pp. 606–610.
- Uhlig, H.H. and R.W. Revie. 1985. *Corrosion and Corrosion Control*, 3rd ed. New York: John Wiley and Sons.
- U.S. Navy. 1988. "Military Handbook: Seawalls, Bulkheads, and Quaywalls." MILHDBK-1025/4.
- Velez, A., G. Gazetas, and R. Krishnan. 1983. "Lateral Dynamic Response of Constrained Head Piles." *Journal of Geotechnical Engineering*, Vol. 109, No. 8, pp. 1063–1081.
- Vesic, A.S. 1973. "Analysis of Ultimate Loads of Shallow Foundations." *Journal of Soil Mechanics and Foundation Design*, Vol. 99, No. SM 1, pp. 45–73.

- Vickars, R.A. and S.P. Clemence. 2000. "Performance of Helical Piles with Grouted Shafts." In *New Technological and Design Developments in Deep Foundations*, pp. 327–341. Reston, VA: American Society of Civil Engineers.
- Victor, R. and A. Cerato. 2008. "Helical Anchors as Wind Tower Guyed Cable Foundations." In *Proceedings of the BGA International Conference on Foundations*. Dundee, Scotland: HIS BRE Press.
- Vyazmensky, A.M. 2005. Numerical Modeling of Time Dependent Pore Pressure Response Induced by Helical Pile Installation." Masters. thesis, University of British Columbia, Vancouver.
- Walsh, K.D. et al. 2009. "Method for Evaluation of Depth of Wetting in Residential Areas", *Journal of Geotechnical and Geoenvironmental Engineering*, February Edition, ASCE, pp. 169–176.
- Weech, C.N. 2002. "Installation and Load Testing of Helical Piles in Sensitive Fine-Grained Soil." Master's thesis, University of British Columbia, Vancouver.
- Weikart, A.M., and S.P. Clemence. 1987. "Helix Anchor Foundations—Two CaseHistories." *Foundations for Transmission Line Towers*, pp. 72–80. Reston, VA: American Society for Civil Engineers.
- Wesolek, D.A., F.C. Schmednecht, and G.L. Seider. 2005. "Helical Piers/Anchors in the Chicago Building Code." In *Proceedings of the 30th Annual Conference on Deep Foundations*. Chicago, IL
- Witherspoon, T.W. 2006. *Underpinning Systems in Expansive Clay Environment*, Ph.D. diss., University of Texas at Arlington.
- Yokel, F.Y., R.M. Chung, and C.W.C. Yancey. 1981. "NBS Studies of Mobile Home Foundations." U.S. National Bureau of Standards Report NBSIR 81-2238. Gaithersburg, MD: NIST.
- Zaki, A. 2006. *Principles of Corrosion Engineering and Corrosion Control*. Oxford, U.K: Butterworth-Heinemann.
- Zhang, D.J.Y. 1999. "Predicting Capacity of Helical Screw Piles in Alberta Soils." Master's thesis, University of Alberta, Edmonton.